

3002 Del Prado Blvd. S. • Ste. B Cape Coral, FL 33904

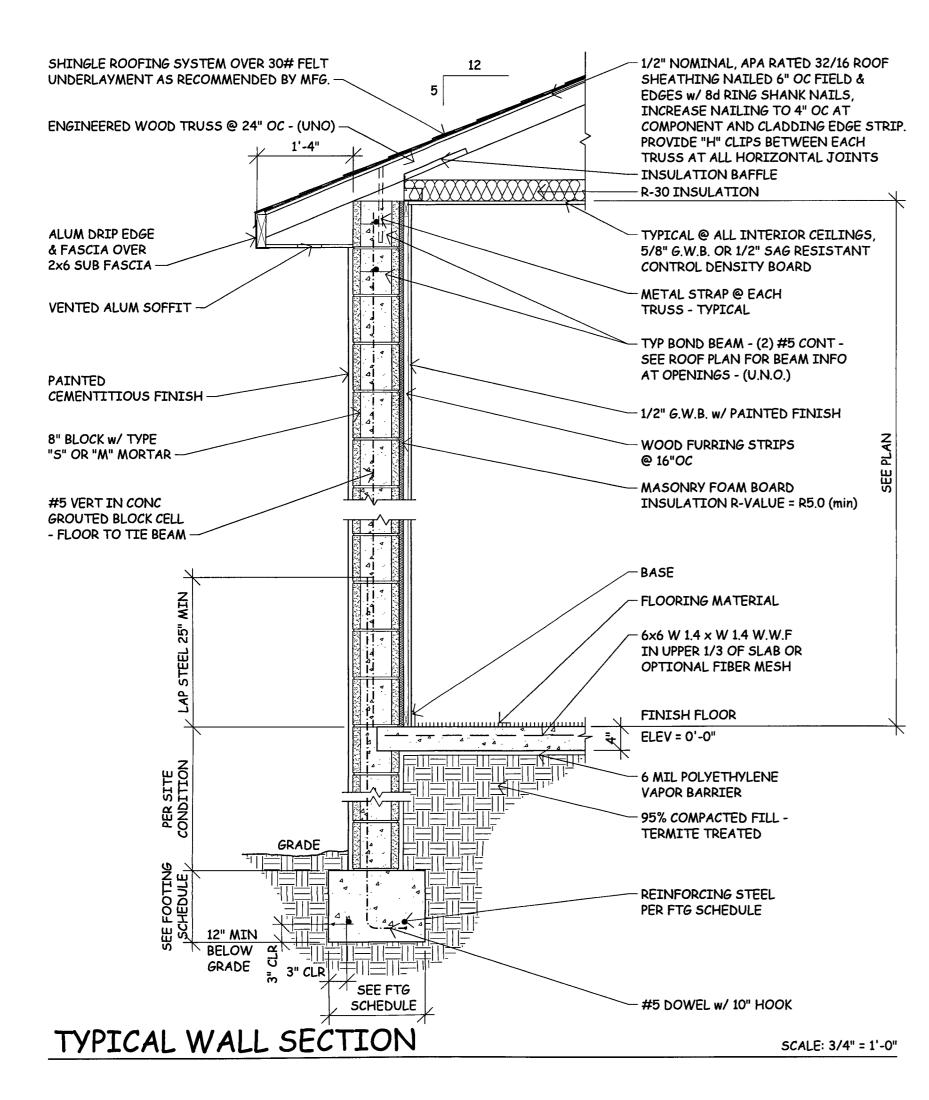
Toll Free: 877-740-3366

Phone: 239-458-6633 • Fax: 239-458-6733 Florida, Certificate of Authorization #9910

Florida, QB66649

CBC059991

CERTIFICATION INFORMATION C		*Smoke Detectors must be labeled in accordance with UL217.		
Contractor:	MMC NATIONAL			
Model:	CHEYENNE MODEL	R703.1.1 SOFFIT COMPLIA	NCE- ALL SOFFITS SHALL BE CAPABLE	
Job Number:	20-4236	OF RESISTING THE DESIGN	PRESSURES SPECIFIED IN TABLE	
Address:	1721 SAVONA PKWY WEST	R301.2(2) FOR WALLS -58.	9 + 43.8 PSF SOFFIT	
Subdivision:	CAPE CORAL			
Lots:				
Block:		single-ply gypsum hoard nail wit	th 6d nails @ 8" O.C. shall be applied	
Unit:			spaces @ 16" O.C. 2: Garage ceilings	
County:	LEE	to have 5/8" single-ply gypsum i		
Engineering Sheets:	LLC	perpendicular to ceiling framing		
Plans/Drafting Sheets:	SEE PLAN	perpendicular to centing training		
R309.1.1 DUCT PENETRAT		R302.1 SEPARATION REST	PICTIONS	
DUCTS IN THE GARAGE AN		K3UZ.1 SEPARATION REST	RICTIONS	
THE WALLS OR CEILINGS S		INCLUDING EXTERIOR WAI	LS/OVERHANGS SEPARATED BY LES	
DWELLING FROM THE GA			LESS THAN 1 HOUR FIRE RESISTIVE	
	NO. 26 GAGE SHEET STEEL	RATING WITH EXPOSURE O	ON BOTH SIDES.	
OR OTHER APPROVED MA	ATERIAL AND SHALL HAVE			
NO OPENINGS INTO GARA	AGE.		•	
		* R302.5.1 Self-closing doo	r between garage and Residence.	
MANUFACTURERS: Provide	ded by Contractor			
	· .			
Doors:				
Windows:				
Overhead Doors:			, pprocupe	
Mitered Glass:		WINDOW & DOOR DESIGN	N PRESSURE	
Sliding Glass Doors:	CEE BLAN	IF WORST CASE		
Roof Coverings:	SEE PLAN			
		Mean Roof Height:	ft	
GENERAL BUILDING INFO	RMATION:	Design Wind Velocity:	160 mph	
		Windows:	-49.6 + 34.7 psf	
Proposed Floor Area:	SEE PLAN	Doors:	-49.6 + 34.7 psf	
Proposed Roof Ht:	15 ft	Sliding Glass Doors:	-49.6 + 34.7 psf -29.0 + 25.3 psf	
Proposed # Stories:	SEE PLAN	Garage Doors:	-29.0 + 25.3 psf	
Living:	SEE PLAN	Construction Type V-B		
Garage:	SEE PLAN	Enclosed Structure		
Entry:	SEE PLAN	Enclosed Structure		
Storage:	· N/A			
Other:	N/A			
Total OY CH	SEE PLAN			
THE LOT CHA	1/2//		,	
The Abildine Astructure kg	been designed in accordance with the 20	17 6th Edition Florida Building Code	s	
and section 1609 for de	sign agessures generated by a design wind			
No. 72152	X			
* * *	1 1 ★ E			
EN STATE OF	110=	3/18/2	020	



CODE COMPLIANCE VERIFICATION

48"x48"x12"D PAD

DRAWING INDEX

- A1 FOUNDATION PLAN A2 FLOOR PLAN
- A3 ELEVATIONS
- A4 ELEVATIONS
- A5 ROOF PLAN E1 ELECTRICAL PLAN
- ST SITE PLAN
- S1 STRUCTURAL DETAILS S2 STRUCTURAL DETAILS

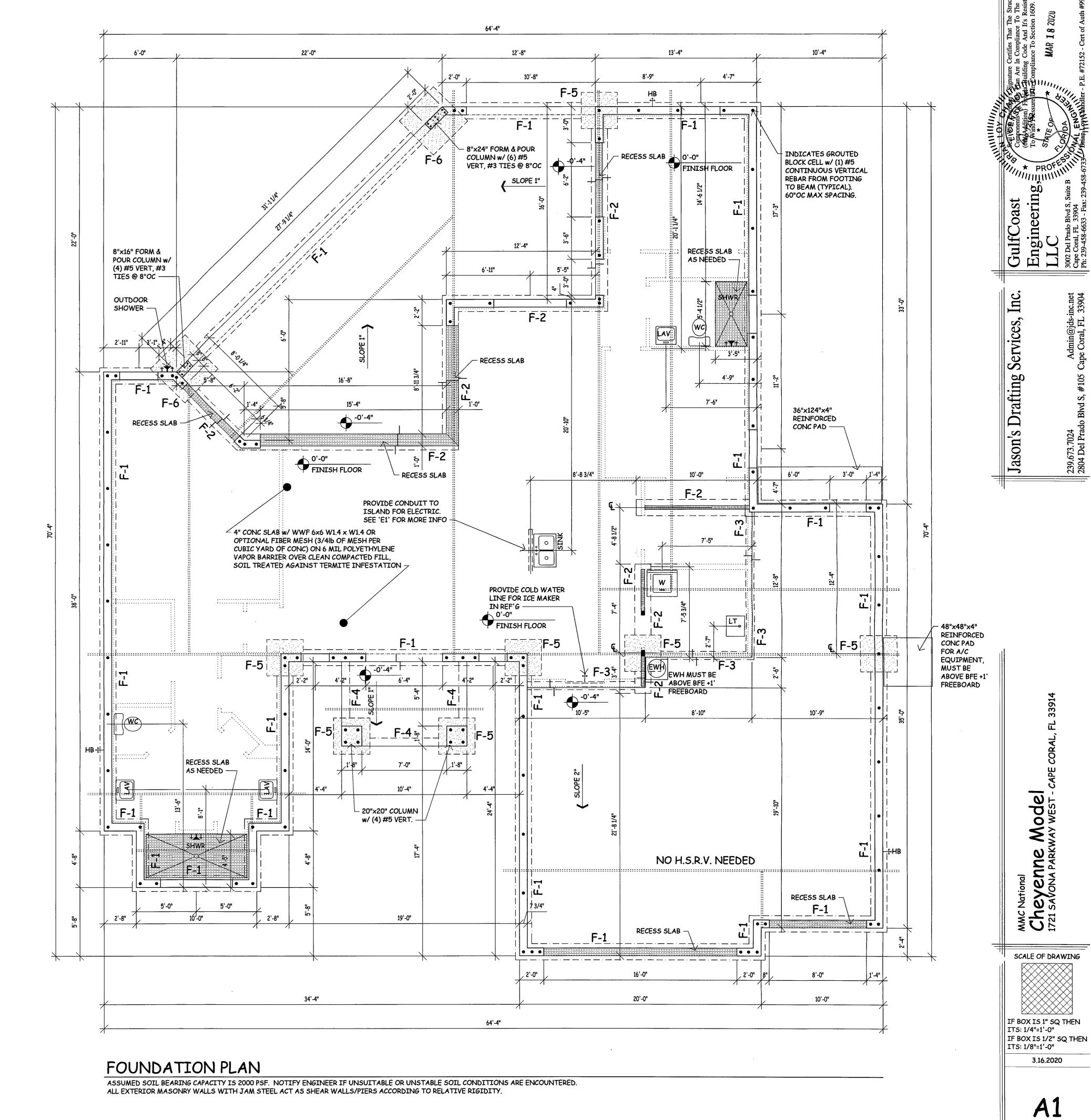
= PAD FOOTING CONDITION

= RECESSED SLAB

NOTE: FINISHED INTERIOR DIM OF SHWR TO BE MIN 30" PER CODE

BUILDING	CODE:		FLORIDA BUILDING CODE, RESIDENTIAL 2017 (6th EDITION) FLORIDA BUILDING CODE			
MECHANI	CAL CODE :	FLORIDA	FLORIDA BUILDING CODE, MECHANICAL 2017 (6th EDITION)			
PLUMBING	S CODE :	FLORIDA	BUILDING CODE, PLUMBIN	NG 2017 (6th EDITION)		
ELECTRIC	AL CODE :	NEC 2014				
FIRE COD	E:	FLORIDA	FIRE PREVENTION CODE 2	2017 (6th EDITION)		
ACCESSIE	BILITY:	FLORIDA	BUILDING CODE, BUILDIN	NG 2017 (6th EDITION)		
ENERGY C	ODE:	FLORIDA	BUILDING CODE, BUILDIN	NG 2017 (6th EDITION)		
WIND	DESIGN					
BASIC WI	IND SPEED	160 MPH (3 SECOND GUST)			
IMPORTA	NCE FACTOR	1.0				
EXPOSURI	E	В				
WETHOD	OF DESIGN	ASCE7-10				
MEAN RO	OF HEIGHT	15'-2"±	15'-2"±			
	L PRESSURE		OSED MAIN BUILDING			
COEFFICI	ENT	0.55 LAN/	AI AND ENTRY			
BUILD	ING DESIG	SN				
RISK CAT	EGORY (2)					
FOO	TING S	CHEDUI	LE	#"W = FOOTING WIDTH #"D = FOOTING DEPTH		
LOCATE	SIZE	ТУРЕ	REINFORCEMENT	COMMENTS		
F-1	16"W × 10"D	STEMWALL	(2) #5 CONTINUOUS	TYP FOOTING		
F-2	16"W × 12"D	INTEGRAL	(2) #5 CONTINUOUS	-		
F-3	8"W x 12"D	INTEGRAL	(1) #5 CONTINUOUS	-		
F-4	8"W × 8"D	ED <i>G</i> E	(1) #5 CONTINUOUS	-		
F-5	36"×36"×12"D	PAD	(4) #5 EACH WAY	-		

(8) #5 EACH WAY



20-4236

DOOR SCHEDULE

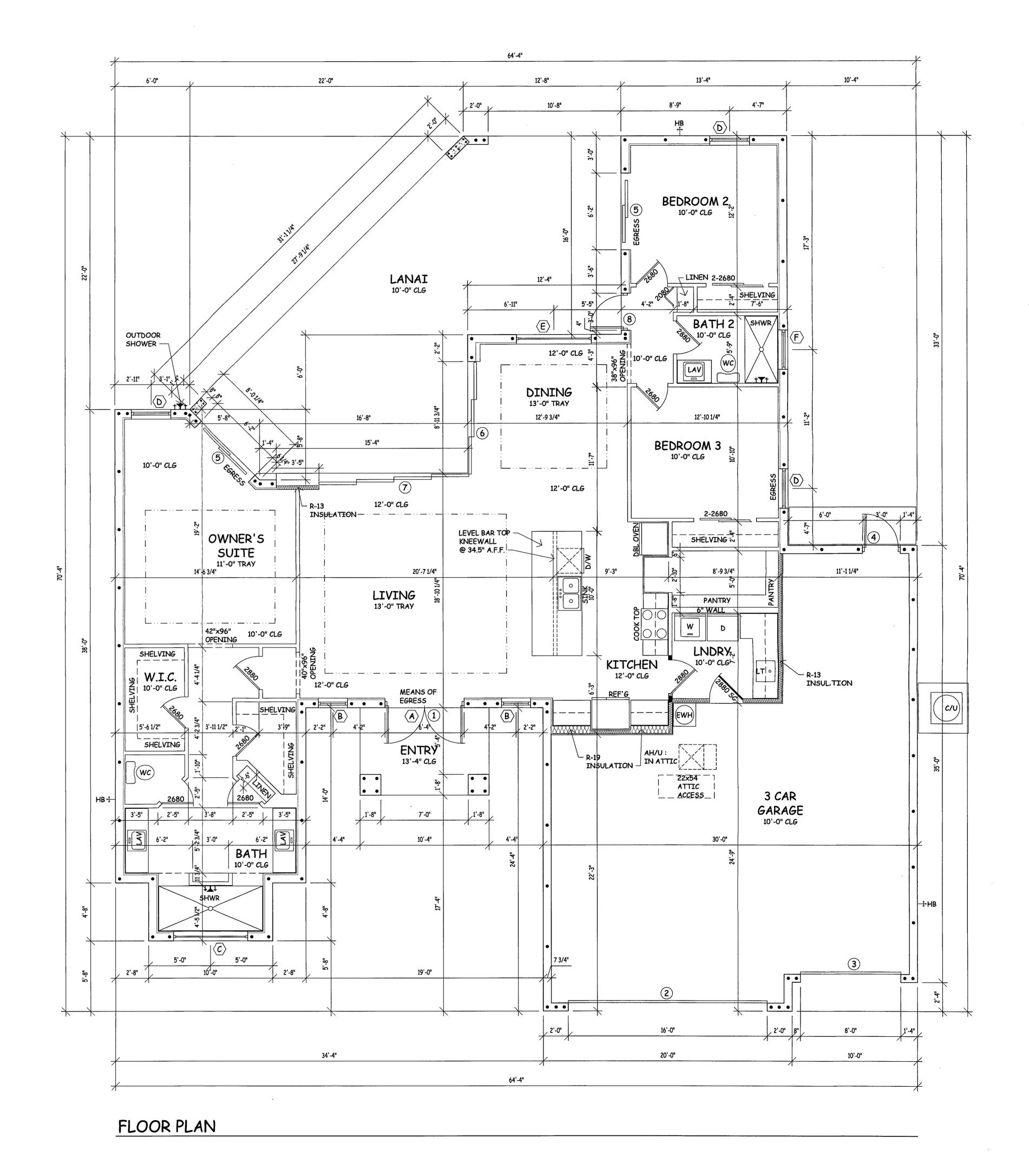
LOCATE	SIZE	MANUFACTURER	NOA # / FL #	HURR. PROTECT	NOTES
1	(2) 3080	PLASTPRO	FL15215.17	IMPACT	FULL LITE - ENTRY
2	16080	CLOPAY	FL16546.4	IMPACT	OVERHEAD
3	8080	CLOPAY	FL16107.24	IMPACT	OVERHEAD
4	2880	PLASTPRO	FL17184.5	IMPACT	SOLID
5	6080	PGT	FL251.4	IMPACT	SLIDING GLASS DOOR
6	9080	PGT	FL251.4	IMPACT	SLIDING GLASS DOOR 90° CORNER
7	12080	PGT	FL251.4	IMPACT	SLIDING GLASS DOOR 90° CORNER - PKT
8	2880	PLASTPRO	FL15215.17	IMPACT	FULL LITE

NOTE: CONTRACTOR TO VERIFY MASONRY OPENINGS PRIOR TO COMMENCEMENT OF WORK.

NOTE: AT LEAST ONE BATH TO BE AN ACCESSIBLE BATH

NOTE: FINISHED INTERIOR DIM OF SHWR TO BE MIN 30" PER CODE

AREAS	
LIVING	1826 SF
LANAI	557 SF
ENTRY	72 SF
GARAGE	829 SF
TOTAL:	3284 SF



n's Drafting Services, Inc.

Engineering, = **

CulfCoast

Engineering, = **

LLC

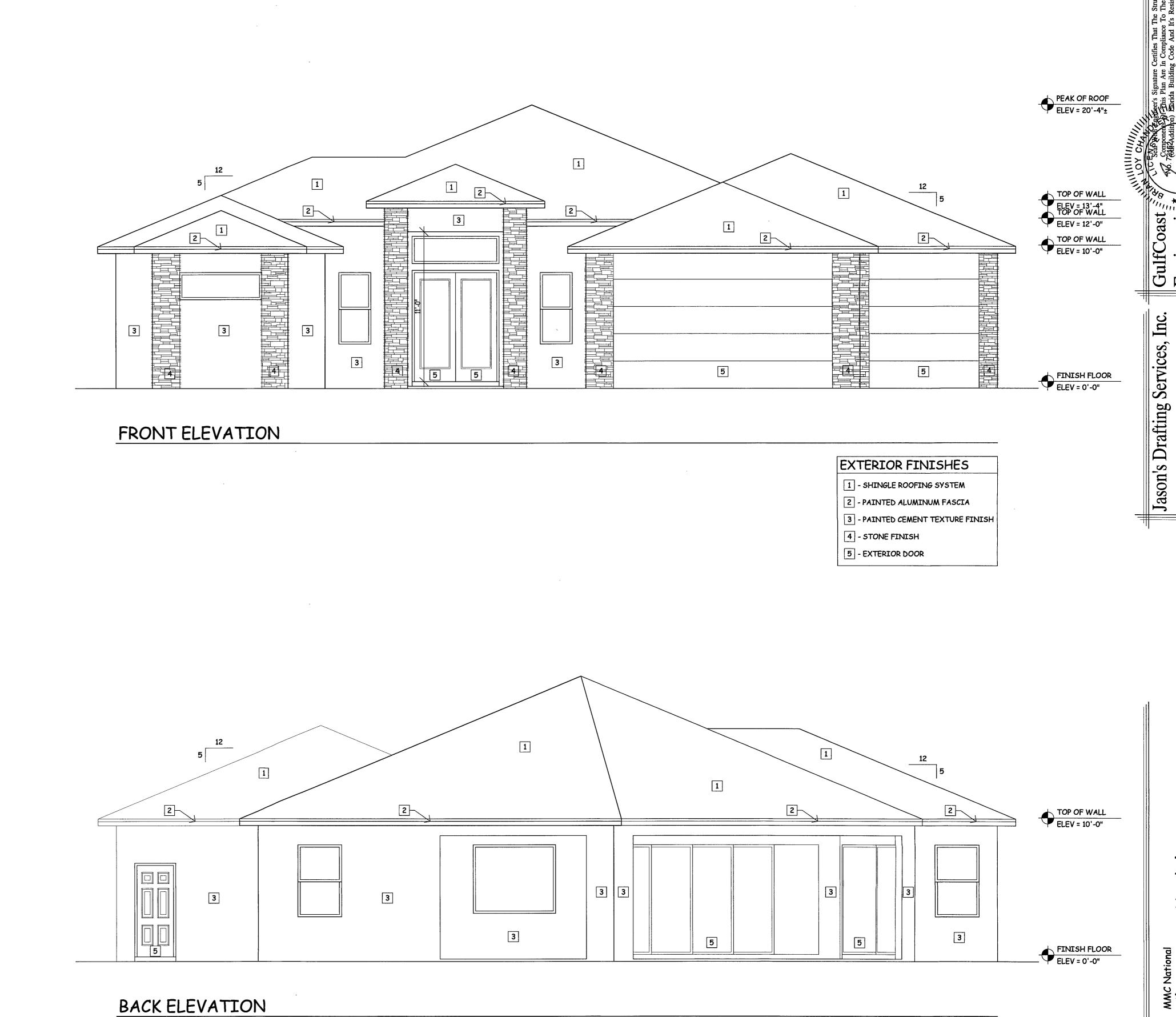
State Corel Fit 33904

Cape Corel Fit 33904

MMC National **Cheyenne Model** 1721 SAVONA PARKWAY WEST - CAPE CORAL, FL 33914

IF BOX IS 1" SQ THEN
ITS: 1/4"=1'-0"
IF BOX IS 1/2" SQ THEN
ITS: 1/8"=1'-0"
3.16.2020

A220-4236



MMC National

Cheyenne Model

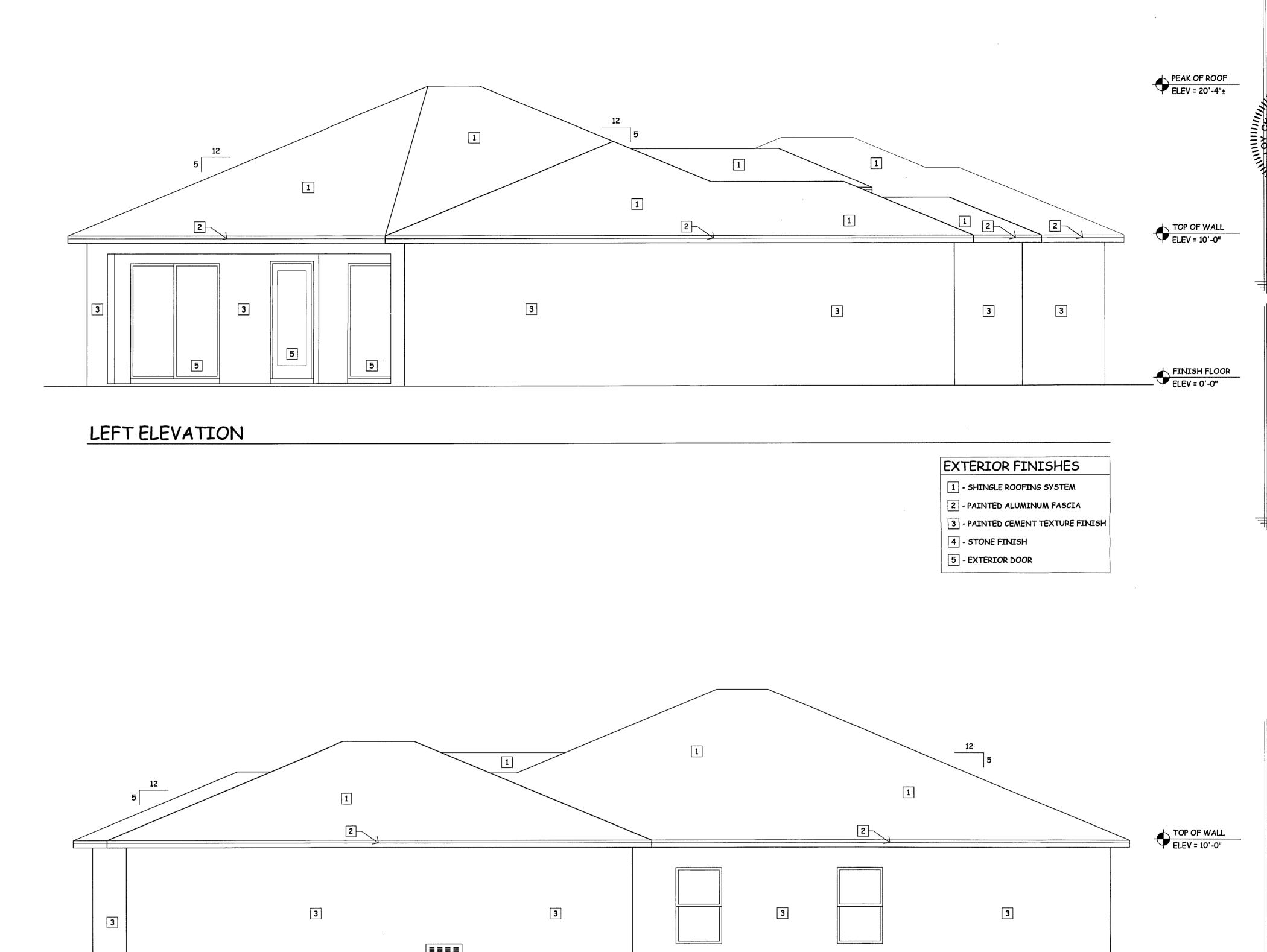
1721 SAVONA PARKWAY WEST - CAPE CORAL, FL 339

SCALE OF DRAWING

IF BOX IS 1" SQ THEN
ITS: 1/4"=1'-0"
IF BOX IS 1/2" SQ THEN
ITS: 1/8"=1'-0"

3.16.2020

A3



RIGHT ELEVATION

MMC National

Cheyenne Model

1721 SAVONA PARKWAY WEST - CAPE CORAL, FL 33914

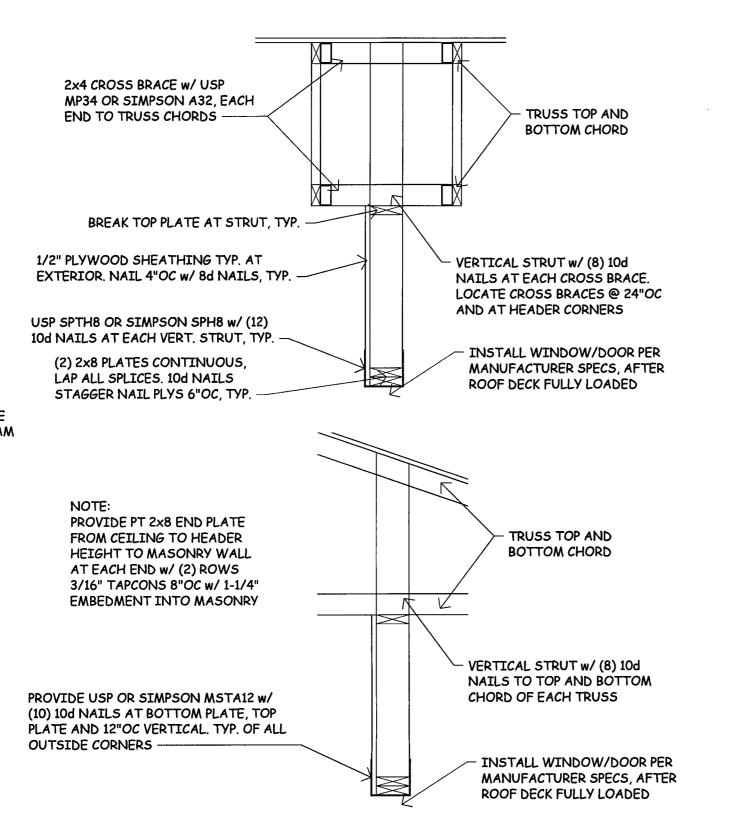
FINISH FLOOR

ELEV = 0'-0"

IF BOX IS 1" SQ THEN
ITS: 1/4"=1'-0"
IF BOX IS 1/2" SQ THEN
ITS: 1/8"=1'-0"

3.16.2020

A4 20-4236 CONCRETE BEAM CONNECTION

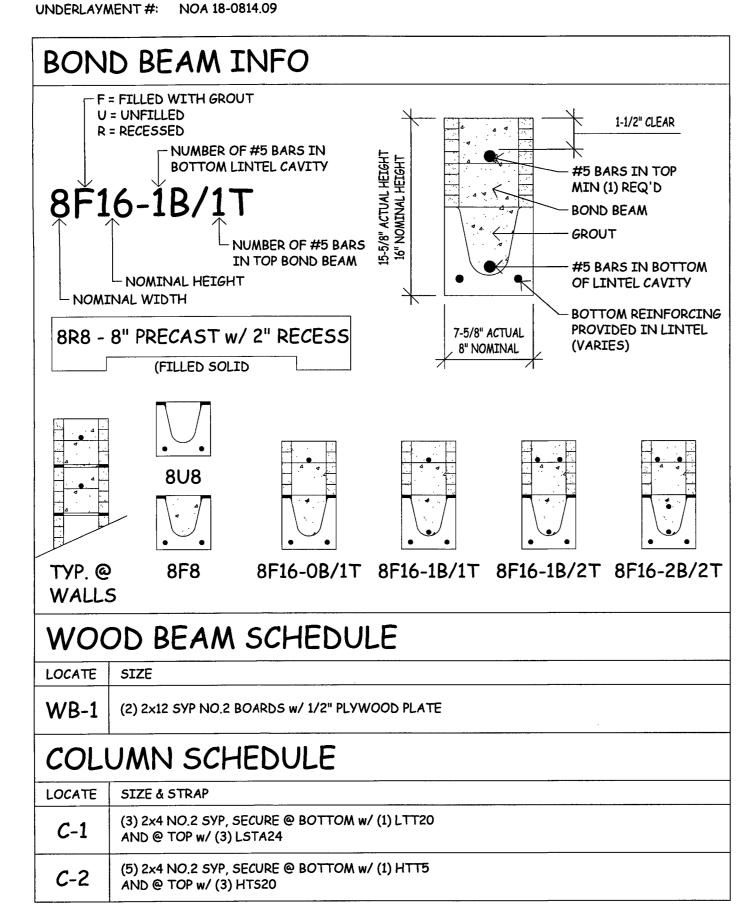


SCALE: 3/4" = 1'-0"

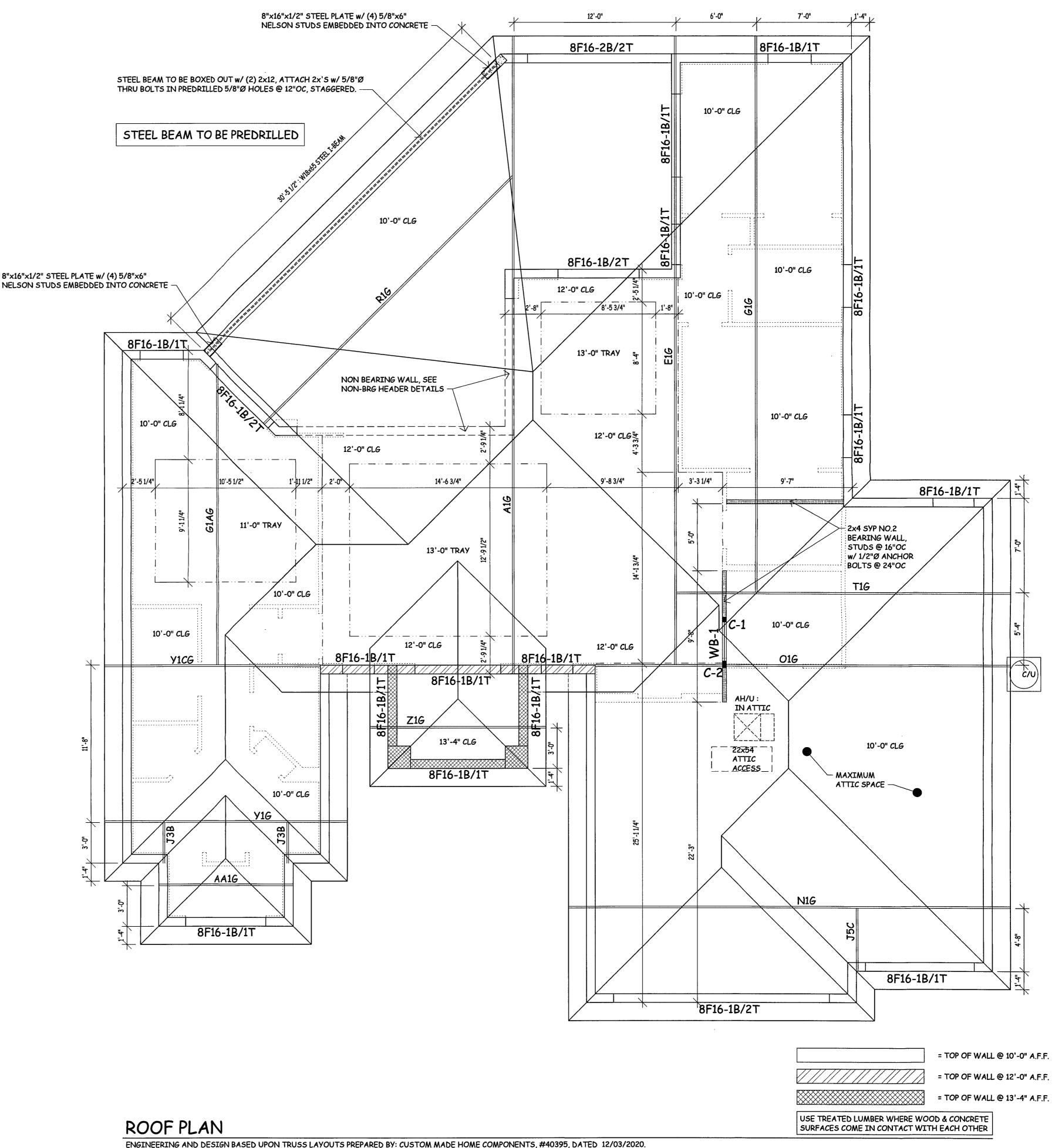
ATTIC VENTILATION CALCULATION

NON-BRG HEADER DETAILS

PER FBC 2017 6th EDITION PER R806.1 3702 sqft OF ROOF AREA 3702 / 150 =24.68 sqft X 144 =3553.92 sqin OF TOTAL VENTING REQUIRED. LOWER VENTILATION: 22.3 sqin PER LINEAR FOOT AT 333.3 FEET = 7432.59 FL# 12194 ROOFING MATERIAL #: NOA 16-0329.13



EACH COLUMN FOR HEADER OPENING SHALL CONSIST OF (1) KING STUD RUNNING FULL HEIGHT OF WALL ADJACENT TO HEADER AND ALL REMAINING STUDS SHALL BE JACK STUDS THAT TERMINATE UNDER THE HEADER.



IF MANUFACTURER'S LAYOUT DRAWINGS DIFFER IN CONFIGURATION OR BEARING POINTS FROM THIS PLAN, IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO

HAVE SAID TRUSS MANUFACTURER'S DRAWINGS REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER OF RECORD, BEFORE THE CONSTRUCTION OF THE ROOF SYSTEM.

MMC National

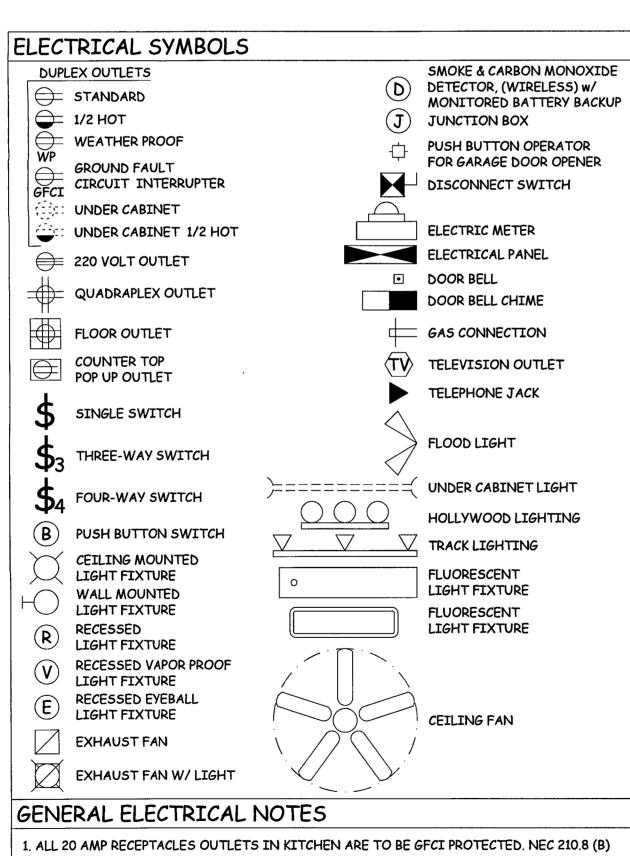
Cheyenne Model

1721 SAVONA PARKWAY WEST - C.

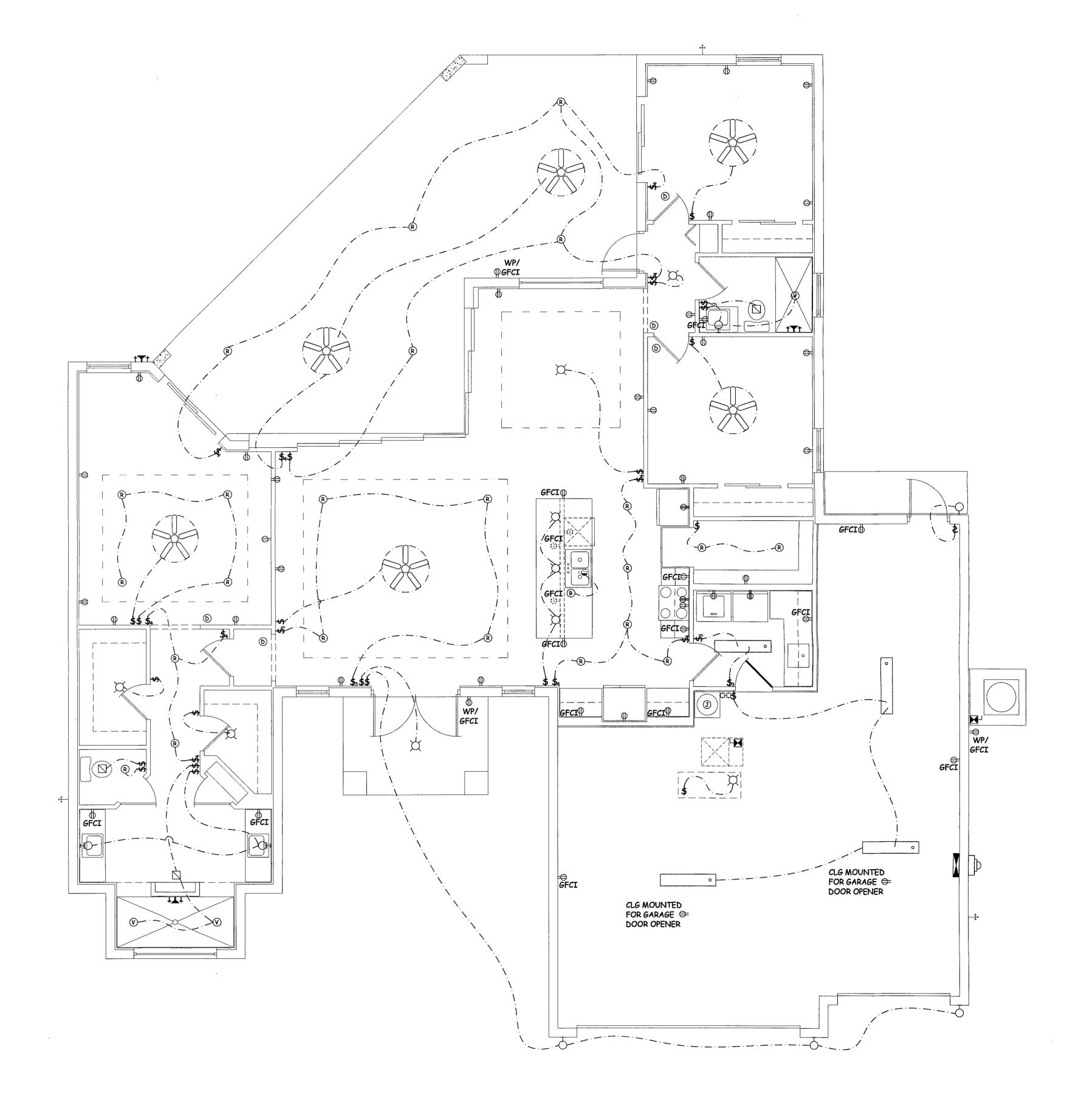
IF BOX IS 1" SQ THEN ITS: 1/4"=1'-0" IF BOX IS 1/2" SQ THEN ITS: 1/8"=1'-0" 3.16.2020

SCALE OF DRAWING

A5



- 1. ALL 20 AMP RECEPTACLES OUTLETS IN KITCHEN ARE TO BE GFCI PROTECTED. NEC 210.8 (
 2. BEDROOM OUTLETS SHALL BE PROTECTED BY AN ARC FAULT CIRCUIT INTERRUPTER (AFCI) BREAKER. NEC 210.12 (B)
- 3. ELECTRICAL OUTLETS AND LIGHT FIXTURES TO BE MORE THAN 10' FROM WATER'S EDGE @ POOLS, EXCEPT WHERE A POOL IS WITHIN 3.0m (10ft) OF A DWELLING AND THE DIMENSION OF THE LOT PRECLUDES MEETING THE REQUIRED CLEARANCES, NOT MORE THAN ONE RECEPTACLE OUTLET SHALL BE PERMITTED IF NOT LESS THAN 1.5m (5ft) MEASURED HORIZONTALLY FROM THE INSIDE WALL OF THE POOL. NEC 680.22 (A)(1)(4)
- 4. 15 AND 20 AMP, 125 AND 250 VOLT RECEPTACLES INSTALLED IN A WET LOCATION SHALL HAVE AN ENCLOSURE THAT IS WEATHERPROOF WHETHER OR NOT THE ATTACHMENT PLUG CAP IS INSERTED. NEC 406.8 (B)(1)
- 5. SMOKE DETECTORS ARE NOT PERMITTED WITHIN 3' OF A BATH DOOR OR AN A/C RETURN. NFPA 72 11.8.3.5 (5)



ELECTRICAL PLAN

WWC National

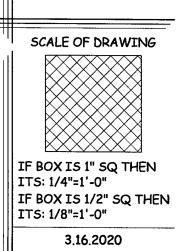
SCALE OF DRAWING

THEN

E1 20-4236

3.16.2020

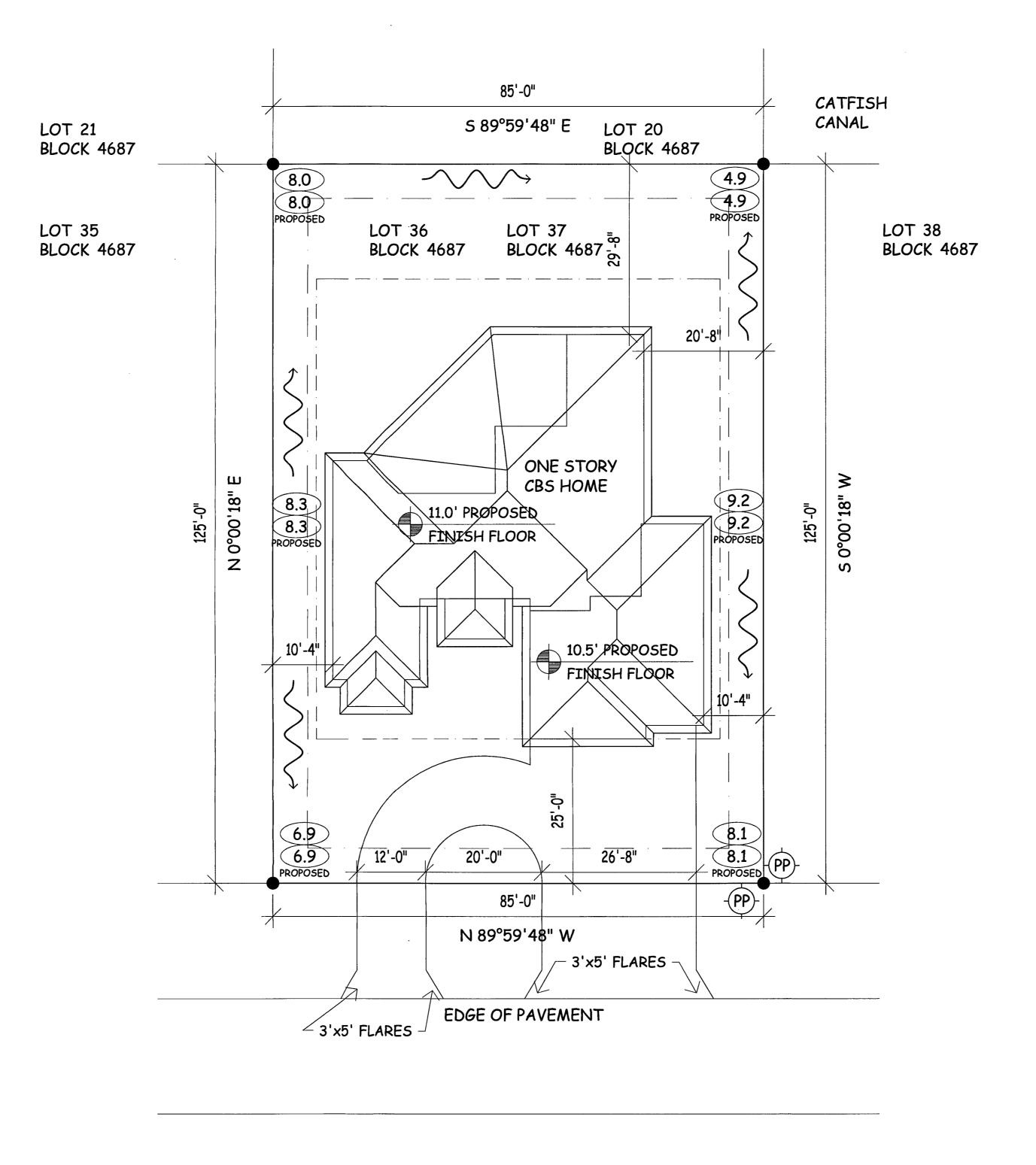
|| ITS: 1/8"=1'-0"



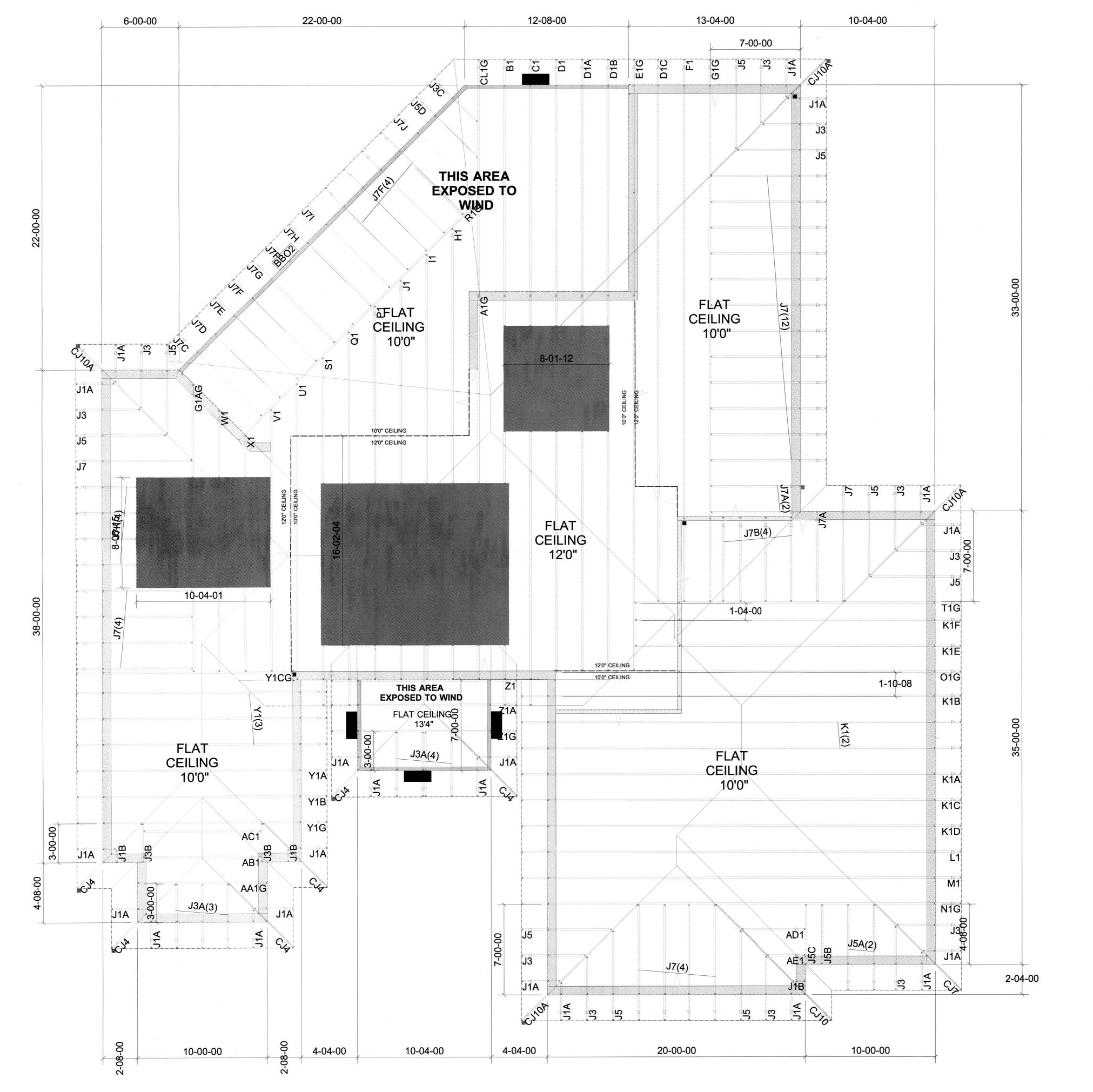
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20-4236

LOT SQFT = 10,625



SAVONA PARKWAY WEST



BUILDING CODE

FBC 2017 LOAD CONDITIONS OCCUPANCY CATEGORY: RESIDENTIAL **EXPOSURE CATEGORY:** WIND DESIGN METHOD: **ASCE 7-10 OPENING CONDITIONS: ENCLOSED** INTERNAL PRESSURE COEFFICIENT:. +/- 0.18 WIND SPEED: 160 MPH

ROOF

HEIGHT ABOVE GROUND:.

CHORD	LIVE	DEAD	TOTAL	DEAD LOAD TO RESIST UPLIFT		
OHORD	LB / SQ. FT. (PSF)					
TOP	20	15	35	5		
воттом	0	10	10	5		
TOTAL	20	25	45	10		

15 FT.

The Truss Manufacturer's Layout appears to be in The Truss Manufacturer's Layout appears to be in substantial compliance with the permitting plans. The contractor must verify all dimensions, pitch, heel heights, overhangs, etc.

2017 6th Edition Florida Building Code Residential GULFCOAST ENGINEERING, LLC COA #9910

BRIAN LOY CHANDLER, P.E. #72152 DATE

MAR 18 2020

CONTRACTOR TO VERIFY DIMENSION DETAILS & SPECIFICATIONS SHOWN. VERIFY WEIGHT & LOCATION OF A.C. UPON ACCEPTING, SIGNING & APPROVING LAYOUT

BY SIGNING THIS LAYOUT YOU ARE AGREEING TO **OUR INTERPRETATION OF THE BLUEPRINT GIVEN** TO US. ANY CHANGES FROM THIS LAYOUT, BY REVISION TO THE BLUEPRINT OR BY NOT REVIEWING THE LAYOUT IN DETAIL ARE SOLELY THE RESPONSIBILITY OF THE BUILDER.

CUSTOM MADE HOME COMPONENTS, INC. WILL BEGIN TO ENGINEER THIS JOB. AT THIS POINT ANY BLUEPRINT CHANGES BY BUILDER ARE AT BUILDERS EXPENSE.

APPROVED BY:

DATE:

DEL DATE:





LANAI & PORCH EXPOSED TO WIND. CHECK PROFILES FOR PORCH LOADING

UPLIFTS AND DOWN LOADS EXCEEDING MAX. LIMITS LISTED BELOW WILL BE NOTED ON LAYOUT AS SHOWN AT BELOW. REACTIONS IN THIS LAYOUT ARE SUPERCEDED BY PROFILE VALUES.

> MAX. UPLIFT = 1000# MAX DOWNLD. = 3500#



	То:				
	Job: 1721 Sav	Job# 40395			
	Pitch	ТС	5/12	OH: 24" PL	All Bearings @ 9'-4" Unless Noted
STOM MADE WE COMPONENTS	I Itori	BC:	xx/12	T.C.: 2x4	All Trusses @ 24" O.C. Unless Noted
sses Built to Last	Drawn By: FF		Dat	te: 12/03/2020	SCALE: 1/4" = 1'-0"

EXTERIOR WALL COMPONENTS OF THE ENCLOSED BUILDING HAS SPECIFIC PRODUCT APPROVAL TO PRESERVE THE ENCLOSED BUILDING ENVELOPE AGAINST IMPACT LOADS AS SET FORTH IN CHAPTER 1). BALUSTERS AT STAIRS, LANDINGS AND BALCONIES SHALL BE SPACED TO PREVENT PASSAGE of a 4" diameter sphere (when required). 10. PROVIDE A MIN. OF 4" CLEAR ALL AROUND AIR HANDLER UNITS, OR AS REQ. BY MFG.

11. IN ZERO LOT LINE HOMES: DRYER VENTS, EXHAUST FANS AND KITCHEN HOODS SHALL NOT VENT THROUGH THE SIDE WALL AND MUST MAINTAIN A 10'-0" SETBACK FROM SIDE PROPERTY LINES.

APPLICABLE CODES

	and the second s
PLANS HAVE BEEN PF	REPARED IN COMPLIANCE WITH THE 2017 SIXTH EDITION CODE AND ASCE 7-
BUILDING CODE	FLORIDA BUILDING CODE, BUILDING 2017 SIXTH EDITION
BUILDING CODE	FLORIDA BUILDING CODE, RESIDENTIAL 2017 SIXTH EDITION
BUILDING CODE	FLORIDA BUILDING CODE, EXISTING BUILDING 2017 SIXTH EDITION
PLUMBING CODE	FLORIDA BUILDING CODE, MECHANICAL 2017 SIXTH EDITION
MECHANICAL CODE	FLORIDA BUILDING CODE, PLUMBING 2017 SIXTH EDITION
ELECTRICAL CODE	NEC 2014 AND NFPA 70
FIRE CODE	2017 SIXTH EDITION FIRE PREVENTION AND NFPA 101
ACCESSIBILITY CODE	FLORIDA BUILDING CODE, ACCESSIBILITY 2017 SIXTH EDITION
ENERGY CODE	FLORIDA BUILDING CODE, ENERGY CONSERVATION 2017 SIXTH EDITION
BASIC WIND SPEED	160 MPH
IMPORTANT FACTOR	1.0
EXPOSURE	В
METHOD OF DESIGN	ASCE 7-10
BUILDING TYPE	ENCLOSED 0.18
RISK CATEGORY	RISK CATEGORY 2

FASCIA AND SOFFIT VENTING NOTES

1. MINIMUM 2" X 4" SUB FASCIA NAILED TO TRUSS TAILS W/(2) NAILS AT EACH TRUSS (EACH PLY WHEN MULTIPLE TRUSS)

2. TYPICAL DRIP EDGE AND SOFFIT/FASCIA INSTALLED TO MFG. SPECIFICATIONS. ENTRY AND LANAI CLG. SPECIFICATIONS

ALL LANAI AND ENTRY CEILING:

MINIMUM - 1/2" NOM C.D.X. PLYWOOD OR OSB FASTENED WITH 8d NAILS AT 4" O.C.

1/2" SAG RESISTANT GYPSUM BOARD OVER 1 X 4 P.T. (OR BETTER) STRIPPING AT 16" O.C. ATTACHED TO TRUSSES WITH (2) 16d NAILS.

LOAD SCHEDULE

TRUSS DESIGN INFORMATION						
<u>LOCATION</u>	LIVE	<u>DEAD</u>	<u>PART'N</u>	<u>TOTAL</u>		
FIRST FLOOR	40	15	10	65		
SECOND FLOOR	40	15	10	65		
ROOF	20	25	0	4 5		

(ROOF LOAD DESIGN SUPPLIED BY TRUSS ENGINEER) SEE ABOVE CHART FOR WIND SPEED CATEGORY, AND DESIGN INFORMATION

THIS STRUCTURE IS DESIGNED IN ACCORDANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE AND ASCE, DEISON PRESSURES FOR EXTERIOR DOORS AND WINDOWS ARE NOTED ON THE ELEVATIONS. WINDOW AND DOOR MANUFACTURERS SHALL MULTIPLY THE GIVEN DESIGN PRESSURES BY THE APPROPIATE FACTORS TO OBTAIN TEST PRESSURES.

1. CONCRETE UNITS TO BE ASTM C 90-030. ASTM GRADES NI OR NIL PROVIDE PRECAST LINTELS AS NECESSARY. ((N = 1500 psi)

BOND BEAMS WHERE SPECIFIED SHALL BE FILLED W/

615–040. PER KEINFORCING SECTION (GRADE 40)
FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL CORE, IT
BE SLOPED MORE THAN ONE HORIZONTAL IN SIX VERTICAL ALIGNMENT,
IGH IT IS IN AN ADJACENT CELL TO THE VERTICAL WALL REINFORCING,
L REINFORCING STEEL SHALL HAVE A MINIMUM CLEARANCE OF ONE—HALF INCH FROM THE MASCRICT.

D. VERTICAL REINFORCING SHALL BE LOCATED AS ON THE PLAN AND AS INDICATED IN THE COLUMN SCHEDULE.

E. VERTICAL REINFORCING EACH SIDE OF ANY OPENING, IF REQUIRED, SHALL BE CONTINUOUS TO THE TIE BEAM. PRECAST LINTELS SHALL HAVE OPENINGS TO ALLOW REINFORCING BARS TO CONTINUE UNINTERRUPTED.

HORIZONTAL REINFORCEMENT SHALL BE PROVIDED AS SCHEDULED IN TH A. HORZONTAL REINFORCEMENT SHALL BE PROVIDED AS SCHEDULED IN THE BEAM SCHEDULE.

B. HORIZONTAL JOINT REINFORCEMENT SHALL CONSIST OF AT LEAST 9 GAGE LADDER TYPE REINFORCEMENT SPACED NOT MORE THAN 16" O.C. VERTICAL REINFORCEMENT SHALL ALSO BE PROVIDED AT THE BOTTOM AND TOP OF ALLOPENINGS, AND EXTEND NOT LESS THAN 24" BEYOND THE OPENINGS, PROVIDE 8" MIN. EMBEDMENT INTO, CONCRETE COLUMNS AND BEAMS AND FULL DEPTH LAPS AT ALL MASONRY "L" AND "T" INTERSECTIONS.

9. SEE REPAIR DETAIL FOR "MISSED" DOWNPOUR LOCATIONS. DETAIL MAY BE USED FOR MAXIMUM OF FIVE (5) LOCATIONS CONSECUTIVELY. MORE THAN FIVE (5) CONSECUTIVE MISSED LOCATIONS WILL REQUIRE ADVISEMENT FROM ENGINEER. 10. ALL MASONRY UNITS ARE TO BE INSTALLED PER THE FLORIDA BUILDING CODE. 11. CONCRETE IS TO BE INSTALLED TO THE LATEST PUBLICATIONS OF THE ACI MANUALS. THE ENGINEER DOES NOT WARRANT THE SLAB, ANY CONCRETE OR ANY MASONRY FROM CRACKING.

1. SCHEDULED HOOPS AND STIRRUPS SHALL BE PLACED AT EACH END OF BEAM OR THROUGHOUT BEAM AS INDICATED ON BEAM SCHEDULE. STIRRUPS SHALL BE TYPE 5-6 AND HOOPS SHALL BE TYPE T-2 TYPICAL CRSI BAR BENDS UNLESS OTHERWISE NOTED. 2. ALL BEAM TOP BARS SHALL BE CONTINUOUS UNLESS OTHEMSE NOTED. 3, ALL TIE BEAMS REINFORCING SHALL BE CONTINUOUS THROUGH TIE-BEAMS ONLY. ALL SPLICES TO BE A MINIMUM OF 40 BAR DIAMETERS. 4. BUNDLE ALL STRUCTURAL TIE BEAM TOP BARS IN PAIRS OVER SUPPORTS WITH TOP BARS FROM ADJACENT BEAMS. (UNLESS NOTED OTHERWISE)

6. TIE BEAM SCHEDULE CEPTH'S ARE MINIMUM AND MAY BE INCREASED 8" TO FIT BLOCK WORK. 7. ALL ADDED LONGITUDINAL BEAM REINFORCING SHALL EXTEND 6" MIN. INTO SUPPORT UNLESS OTHERWISE NOTED.

B. REFER TO CONCRETE NOTES FOR INFORMATION ON CONCRETE AND STEEL SPECFICATIONS. 9. PROVIDE 1 1/2" COVERAGE TOP, SIDES, BOTTOM, AND 1" BETWEEN ADJACENT REBAR

1. FC PRECAST UNTELS = 3500 PSI
2. FC PRESTRESSED LINTELS = 6000 PSI
3. FC GROUT = 3000 PSI W/ MAXIMUM 3/E" AGGREGATE
4. CONCRETE MASONRY UNITS (CMU) PER ASTM C90 W/ MINIMUM
NET AREA COMPRESSIVE STRENGTH = 1900 3SI
5. REBAR PROVIDED IN PRECAST LINTEL PER ASTM A615 GR60
FIELD REBAR PER ASTM A615 GR40 OR CR60
6. PRESTRESSING STRAND PER ASTM A416 GRADE 270 LOW RELAXATION
7. 7/32 WIRE PER ASTM A510
8. MORTAR PER ASTM C270 TYPE M OR S

GENERAL INSTALLATION NOTES 1. ALL VALUES RASED ON MAL (* PEARING 2. SROYCE FULL MORTAR-HEAD AND BED JOINTS. 3. SHORE FILLED LINTELS AS REQUIRED. 3. SHORE FILLED LINTELS AS REQUIRED. 4. INSTALLATION OF LINTEL MUST COMPLY WITH THE ARCHITECTURAL AND/OR STRUCTURAL DRAWINGS. 5. LINTELS ARE MANUFACTURED WITH 5 1/2" LONG NOTCHES AT ENDS TO ACCOMODATE VERTICAL CELL RENFFORCING AND GROUTING. 5. ALL LINTELS MUST MEET OR EXCEED L/360 VERTICAL DEFLECTION, EXCEPT LINTELS 17'-4" AND LONGER WITH A NOMINAL HEIGHT OF 8" MEET OR EXCEED L180. 7. 7/32" DIAMETER WIRE STIRRUPS ARE WELDED TO THE BOTTOM STEEL FOR MECHANICAL ANCHORAGE. 8. BOTTOM FIELD ADDED REBAR TO BE LOCATED AT THE BOTTOM OF THE LINTEL CAVITY. 9. CAST IN PLACE CONCRETE MAY BE PROVIDED IN COMPOSITE LINTEL IN LIEY OF CMU. 10. SAFE LOAD RATINGS BASED ON RATIONAL DESIGN ANAYSIS PER ACI 318 AND ACI 530.

TRUSS BRACING NOTES

1. BRACING SHOWN DOES NOT INCLUDE TEMPORARY BRACING REQUIRED FOR THE ERECTION AND' INSTALLATION OF TRUSSES PRIOR TO THE INSTALLATION OF PERMANENT CROSS BRACING AND WEB LATERAL BRACING REQUIREMENTS. TEMPORARY BRACING IS THE RESPONSIBILITY OF THE TRUSS INSTALLER AND SHOULD REMAIN IN PLACE AS LONG AS NECESSARY FOR THE SAFE AND ACCEPTABLE INSTALLATION OF THE ROCF OR FLOOR. REFER TO HIBBY & BMT-76 2. PERMANENT WEB LATERAL BRACING IS TO BE INSTALLED AS SHOWN ON THE INDIVIDUAL TRUSS DRAWINGS (WEB LATERAL) BRACING IS NOT SHOWN ON THIS SHEET) PERMANENT WEB LATERAL BRACING IS TO BE RESTRAINED TO PREVENT LATERAL MOVEMENT BY SOLID ANCHORAGE TO END WALLS OR BY PERMANENT DIAGONAL BRACING (NOT SHOWN) IN THE PLANE OF THE WEB MEMBER AT INTERVALS NOT TO EXCEED 20" ALONG CONTINUOUS WEB BRACE REFER TO HIB91 & BMT—78 3. PERMANENT DIAGONAL BRACING AS SHOWN ON THIS SHEET DOES NOT INCLUDE BRACING REQUIRES FOR GABLE END STUDS OR BOTTOM CHORD OF CABLE END AT WALL. REFER TO STANDARD CABLE END DETAIL FOR BRACING REQUIREMENTS WHEN REQUIRED.

4. ALL TO? CHORDS ARE ASSUMED TO BE SHEATHED AND ALL BOTTOM CHORDS BRACED BY A RIGIC CRUING SYSTEM. REFER TO INDIVIDUAL TRUSS DRAWINGS FOR BRACING REQUIREMENTS OF NON-SHEATHED TOP CHORDS AND UNSUPPORTED BOTTOM CHORDS.

5. REFER TO TPI / HIB31 & BMT-76 COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLATION AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" APPENDIX C, FOR RECOMMENDED SEQUENCE OF INSTALLING BRACE COMPONENTS. 6. THE STRUCTURAL ADEQUACY OF THE INDIVIDUAL TRUSS DESIGNS TO WITHSTAND GRAVITY AND WIND LOADINGS AS WELL AS GREET CONNECTIONS AND LOADS ARE THE RESPONSIBILITY OF THE TRUSS DESIGNER. ANCHORAGE OF THE TRUSSES AND GROERS TO THE SUPPORTING STRUCTURE TO WITHSTAND WIND UPLIFT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER.

7. THE BRACING FOR THE ROOF SYSTEL IS DESIGNED FOR BOTH POSITIVE AND NEGATIVE PRESSURE AND MEETS THE REQUIREMENTS OF THE FBC $\,$ 8. BRACING SHALL BE A MIN. OF 2"X4" SPF W/(2) 10d NAILS OR 1"X4" SPIB #2 10d NAILS. 9. BOTTOM CHORD BRACING SHALL BE INSTALLED ON TOP SIDE OF BOTTOM CHORL CENTERED BETWEEN BEARING POINTS THEN EQUALLY SPACED 8° -0° OC MAX EACH SIDE — LAPPED 24 MIN. 10. INTERMEDIATE 'X" BRACING REQUIRED FOR TRUSS HEIGHTS 5' AND GREATER ON (10) COMMON TRUSSES OR MORE SPACED AT A MAXIMUM 20' — SEE DETAIL IN HIB91.

11. INSTALL BOTTOM CHORD STACING AT 15 FT. ON CENTER MAXIMUM.
MAXIMUM DISTANCE FROM EDGE OF BEARING SUPPORT TO BOTTOM CHORD BRACE SHALL BE 10' WALL & CLG GYPSUM BOARD APPLICATION NOTES

1. FOR TYPICAL INTERIOR WALLS 1/2" SINGLE PLY GYP. BD. ATT. W/ SCREWS @ 8" O.C. MIN. EMBEDMENT 3/4", SHALL BE APPLIED TO ALL RESIDENCE FRAMING MEMBERS SPACED @ 16" O.C. 2. FOR GARAGE INTERIOR WALLS 1/2" SINGLE-PLY GYP BD. ATT W/ SCREWS & 8" D.C. MIN. EMBEDMENT 3/4", SHALL BE APPLIED TOO ALL GARAGE FRAMING MEMBERS SPACED & 16" O.C.

3. FOR ALL COLING FRAMING MEMBERS, APPLY 5/8" SINGLE PLY GYP. BD. OR 1/7" SAG RESISTANT GYP BOARDS. ATT. W/SCREWS @ 7" O.C. MIN EMBELMENT 3/4" PERPENDICULAR TO CEILING FRAMING. 4. FOR GARAGE CEILINGS BENEATH HAPITABLE ROOMS 5/8" TYPE X
OYPSUN BOARDS CHALL BE APPLIED. ATT. W/ SCREWS 6 6" O.C. MIN EMBEDMENT 3/4"
PERPENDICULAR TO CEILING FRAMING.

CONCRETE NOTES

2. MIN. CONCRETE COVER OVER REINFORCING SHALL BE SLABS ON VAPOR BARRIER BEAMS AND COLUMNS: 1 1/2", FORMED CONCRETE BELOW GRADE: 2", UNIFORMED CONCRETE BELOW GRADE: 3"

5. ADEQUATE VERTICAL AND HORIZONTAL SHORING SHALL BE PROVIDED TO SAFELY SUPPORT ALL LOADS DURING CONSTRUCTION. 6. CONCRETE BEAM SIZES MAY INCREASE (8" MAX) AS REQUIRED FOR ARCHITECTURAL DETAILS OR FIT BLOCK COURSING, DROP BOTTOM OF TIE BEAM AS REQUIRED AT WINDOW AND DOOR BEAMS (28" MAX) AND ADD 2—#5 BARS BOTTOM IF DROP EXCEEDS 8 7. DOWELS COLUMN AND WALL REINFORCING TO FOOTING WITH SAME SIZE AND NUMBER OF DOWELS AS VERTICAL BARS ABOVE.

10. CONCRETE SLABS SHALL NOT BE LOADED UNTIL 12 HOURS HAS ELAPSED. 11, CONCRETE BEAMS SHALL HAVE A MIN. COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS UNLESS OTHERWISE NOTED, ALL CONCRETE PLACEMENTS SHALL BE IN ACCORDANCE WITH ACI 318. 12. CONCRETE IS TO BE INSTALLED TO THE LATEST PUBLICATIONS OF THE ACI MANUALS. THE FINGINEER DOES NOT WARRANT THE SLAB, ANY CONCRETE OR ANY MASONRY FROM CRACKING.

STEEL NOTES

2. WELDED CONSTRUCTION SHALL CONFORM TO THE AMERICAN WELDING SOCIETY "STRUCTURAL WELDING CODE." ELECTRODES FOR FIELD AND SHOP WELDS SHALL BE A.W.S. A5.1E70XX

FOUNDATION NOTES

3. COLUMN AND WALL CENTERLINES SHALL CONCIDE WITH FOOTING CENTERLINES UNLESS OTHERWISE WOTER 6. FILL PLACED WITHIN 5'-0" OF THE CONSTRUCTION PERIMETER SHALL CONSIST OF CLEAN WELL GRADED SAND IN 12" LIFTS (MAX) AND VIBRATORY COMPACTED TO ACHIEVE A MINIMUM OF 95% MCDIFIED PROCTOR ASTM D 1557.

7. AFTER STANDARD CLEANING AND GRUBBING HAS BEEN COMPLETED AND APPROVED, APPLY VIBRATORY COMPACTOR WITH A MINMUM OF FOUR PASSES TO THE EXISTING GROUND. B. CONCRETE SLABS SHALL NOT BE LOADED UNTIL 12 HOURS HAS ELAPSED 9. SPLICES SHALL BE 40 BAR DIAMETERS AND CONTINUOUS AROUND ALL CORNERS AND CHANGES IN DIRECTION. CORNER BARS SHALL BE 40 BAR DIAMETERS EACH WAY. 10. ALL MONO. FOOTINGS HAVE BEEN CHECKED FOR THE USE OF TRANSER REINFORCEMENT AT THE PERIMETER.

12. CONCRETE TESTING, SLUMP TEST, BREAK TEST, QA/QC IS TO BE PERFORMED AT THE DISCRETION OF THE COTNRACTOR, NOT VENIFIED BY ENGINEER UNLESS SPECIFICALLY ARRANGED. 13. PRIOR TO ANY CONSTRUCTIONM THE CONTRACTOR IS TO VERIFY THE EXISTING SITE CONDITIONS PROVIDE A MINIMUM SOIL BEARING CAPACITY PER FBC-R 401, NO GEOTECHNICAL ENGINEERING HAS BEEN PROVIDED BY THE ENGINEER. 14. CONCRETE IS TO BE INSTALLED TO THE LATEST PUBLICATIONS OF THE ACI MANUALS. THE ENGINEER DOES NOT WARRANT THE SLAB, ANY CONCRETE OR ANY MASONRY FROM CRACKING.

TRUSS UPLIFT CONNECTOR SCHEDULE

TRUSS UPLIFT CONNECTOR SCHEDULE: SIMPSON STRONG TIE OR EQUAL 1. THE FOLLOWING TRUSS STRAP SPECIFICATIONS ARE TO BE USED WITH THE PROVIDED MANUFACTURER'S TRUSS PLAN CUIDELINE FOR DESIGNED UPLIFTS. 2. TRUSS STRAPS BELOW ARE PROVIDED TO ALLOW AN INCREASE IN ULIFT STRAPPING IF DESIRED BY CONTRACTORS.

TRUSS UPLIFT	STRAP
0# THRU 1450#	JJETA-20
(451# THRU 1810#	HETA-20
1811# THRU 2500#	2-HETA-20
2501# THRU 5420#	HPA-35 (MULT, TRUSS REO'D)
5421# THRU 10841#	2-HPA-35 (MULTIPLE TRUSS REQ'D
10840# THRU 16620#	2-HD10A
	,
WOOD CONNECTIONS	
TRUSS UPLIFT	STRAP
0# THRU 860#	MTS-20
861# THRU 1245#	HTS20

| 1246# THRU 2490# 2-HTS-20 (SEE NOTE 1 BELOW) | 2491# THRU 4980# 4-HTS-20 (SEE NOTE 2 BELOW) | 4350# THRU 8700# 2-MST-60 1. REQUIRES SPECIAL WALL CONSTRUCTION. SEE STRAP NAILING SPECIFICATION CHART FOR STRAP INSTALLATION AND WALL DETAILS. 2. REQUIRES MULTIPLE PLY TRUSS AND BEARING POST/COLUMN. ATTACH (1) STRAP EACH SIDE EACH FACE OF TRUSS TO BEARING POST/COLUMN

TRUSS STRAP REPAIR DETAIL

| 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# | 175# |

ATTIC VENTILATION REQUIREMENTS

THE TOTAL NET FREE VENTILATING AREA SHALL NOT BE LESS THAN 1 TO 150 OF THE AREA OF THE SPACE VENTILATED. 1. SOFFIT VENTILATION COMPLIANCE WITH FBC IS THE RESPONSIBILITY OF THE CONTRACTOR. 2. SPECIFIC PRODUCT SELECTION FOR EAVE/SOFFIT VENTING AND CALCULATIONS ARE TO BE PROVIDED BY CONTRACTOR.

PLUMBING NOTES

ALL PLUMBING FIXTURES SHALL BE INSTALLED IN ACCORDANCE WITH SECTION 405, FBC 2. ALL PLUMBING FIXTURES SHALL MEET THE REQUIREMENTS OF TABLE 604.3, FBC. 3. DISPOSITION OF RAIN WATER SHALL COMPLY WITH FBC PLUMBING CODE CHAPTER 11. 4. ALL PLUMBING PENETRATIONS MUST BE A MIN. 10'-0" FROM ROOF JACKS. 5. CONDESATE DRAINS MUST BE A MIN. OF 12'-0" OFF THE BUILDING. 6. SHOWER COMPARTMENTS AND WALLS ABOVE BATH TUBS WITH SHOWER HEADS SHALL BE FINISHED WITH A SMCOTH, NONABSORBENT SURFACE TO A HEIGHT NOT LESS THAN 70° ABOVE THE DRAIN INLET.

. ALL HEATING, AIR CONDITIONING, REFRIGERATION AND VENTILATION EQUIPMENT SHALL CONFORM TO THE REQUIREMENTS SET FORTH IN THE FLORIDA BUILDING CODE "MECHANICAL" AND CHAPTER 28 OF THE FLORIDA BUILDING CODE.

3. ALL DWELLING ROOMS MUST HAVE HVAC RETURNS MAY BE DUCTED, JUMPERS OR

GENERAL FRAMING NOTES

THE TRUSS AND FLOOR SYSTEM LAYOUT SHOWN ON THIS SHEET IS SCHEMATIC IN NATURE HOWEVER, THE SUPPORTING SUPERSTRUCTURE HAS BEEN DESIGNED UNDER THE ASSUMPTION THAT THE FRAMING SCHEME SHOWN SILL CLOSELY PARALLEL FINAL TRUSS AND FLOOR SYSTEM

can be modified only after obtaining permission from the prime professional of record WHO MUST REVIEW PROPOSED CHANGES AND AUTHORIZE STRUCTURAL REVISIONS ACCORDINGLY. 3. Final signed and sealed engineered truss and floor system design must be submitted

TO THIS OFFICE FOR REVIEW. TRUSS AND FLOOR SYSTEM DESIGNER MUST PROVIDE ALL TRUSS CONNECTIONS AS PART IF THIS DESIGN. 4. TRUSS AND FLOOR SYSTEM MANUFACTURER SHALL SUBMIT THREE (3) COPIES OF SHOP DRAWINGS AND ENGINEERING CALCULATIONS SIGNED AND SEALED BY A FLORIDA REGISTERED engineer, of their design for review including two (2) copies for engineer's review PRIOR TO FABRICATION REVIEW OF SHOP DRAWINGS IS A COURTESY BY THE ARCHITECT/ENGINEER AND IN NO WAY RELIEVES THE MANUFACTURER OF HIS RESPONSIBILITY TO PROVIDE A LAYOUT THA FULLY INTEGRATES WITH THE ARCHITECT/ENGINEER'S SUPERSTRUCTURE. IF THERE IS ANY CONFLIC

OR ADDITIONAL STRUCTURE NEEDED, THÉ TRUSS AND FLOOR SYSTEM COMPANY SHALL NOTIFY THE ARCHITECT/ENGINEER IN WRITING OF THE SPECIFIC AREAS OF CONCERN. . Trusses to be designed to carry loads of attic ahu and Misc. Equipment, coordinate OCATIONS WITH BUILDER PRIOR TO FABRICATION AND INDICATE ON TRUSS DRAWINGS. THE BUILDER SHALL BE RESPONSIBLE FOR THE COMPLETE COORDINATION BETWEEN THE TRUSS DESIGN

FOOTING UNTIL REVIEW OF APPROVED TRUSS AND FLOOR SYSTEM DRAWINGS, SIGNED AND SEALED BY A FLORIDA REGISTERED ENGINEER. ALL INTERIOR LOAD BEARING PARTITIONS TO BE CONSTRUCTED WITH CLIPS, TOP AND BOTTOM 3. STRUCTURAL WOOD AND TIMBER FRAMING SHALL CONFORM TO THE "TIMBER CONSTRUCTION Manual" as published by the american institute of timber construction. . ALL WOOD IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE TREATED. PROVIDE

IN APPROVED MOISTURE VAPOR BARRIER BETWEEN THE CONCRETE OR OTHER CEMENTITUOUS Material and the wood as per code. BUILDER SHALL BE RESPONSIBLE FOR PROVIDING ADEQUATE BRACING AND BRIDGING USE during erection of trusses and structural slabs to prevent collapse or damage. 11. Interior non-bearing walls are to be constructed using good construction practice and contractor may use any material permitted by the current edition of FBC SUBJECT TO APPROVAL OF THE LOCAL BUILDING AUTHORITY. 12. SEE GENERAL FRAMING NOTES AND HEADER SCHEDULE & REQUIREMENTS FOR SPECIFIC

FRAMING REQUIREMENTS. 13. UNLESS SPECIFIED OTHERWISE, ALL BEARING, SHEAR AND EXTERIOR NON-BEARING WOOD FRAMING WALLS, TRUSS WIND BRACING, BEAMS AND HEADERS SHALL BE DOUGLAS FIR-LARCH, HEM-FIR OR SPRUCE-PINE-FIR (OR EQUAL) CONSTRUCTION GRAD OR BETTER Fb=850psi, Ft=375psi, Fv=70psi, Fc=335psi, Fcil=1,200psi, E=1,000,000psi) WHEN SPECIFIED SOUTHERN PINE SHALL BE NUMBER 2 GRADE OR BETTER (Fb=975psi, Ft=550psi, Fv=90psi, Fc=480psi, Fcil=1,350psi, E=1,400,000psi)

TRUSS. SEE TRUSS MFG. TRUSS PROFILES FOR DEFLECTION SPECIFICATIONS. 5. RUN TOP PLATE CONT. OVER STUDS 10 WALL, BLOCK ALL SPLICES BETWEEN STUDS. 16. IF FULL SIZED HEADER IS USED IN LIEU OF CRIPPLE STUDS, NO STRAPPING REQUIRED EXCEPT AT ENDS. NAIL BOTTOM PLATE TO HEADER W/ (2) ROWS OF 16d NAILS \bullet 8° O.C. 17. CRIPPLE STUDS @ 16" O.C. MAX. (IF NECESSARY) FASTEN W/(1) SIMPSON LSTA24 STRAP. ATTACH UP FACE, OVER TOP PLATE AND NAIL OR (1) STRAP TOP AND BOTTOM FOR WALL HEIGHTS OVER 8"-0".

18. THE DESIGN OF ALL PRE—ENGINEERED ROOF TRUSSES INCLUDING GIRDERS, FLOOR TRUSSES AND ALL BEAMS ARE TO BE DESIGNED TO MEET THE 2017 FLORIDA BUILDING CODE SIXTH EDITION AND ASCE7—10. THE DESIGN IS TO INDICATE THE FLORIDA REGISTERED ENGINEER WHO DESIGNED THEM AND BEAR THE SEAL OF SUCH ENGINEER. ALL LATERAL AND CROSS BRACING REQUIRED IS TO BE SPECIFIED BY THE DESIGNER. THE TRUSS OR FLOOR SYSTEM DESIGN SHALL NOT EXERT LATERAL LOADS ON ANY WALL SYSTEM, INTERIOR OR EXTERIOR. THE DESIGN IS TO ALSO INDICATE THE MAGNITUDE OF THE LOADS AND ANY PROVISIONS REQUIRED. THE CONTRACTOR ASSUMES THE RESPONSIBILITY OF REVIEW OF THE PRE-ENGINEERED SYSTEMS AND ANY COMPLIANCE NECESSARY. ANY DEVIATION FROM THE PROPOSED DESIGNS MAY REQUIRE ADDITIONAL REVIEW

9. ALL PERMANENT TRUSS BRACING, IN ADDITION TO TRUSS BRACING SPECIFIED BY THE TRUSS ENGINEER SHALL BE INSTALLED PER THE DETAIL IN THESE SHEETS, AND IN ACCORDANCE TO

HEADER SCHEDULE (2X4 WALLS) (UNLESS STAT

ATED OTHERWISE ON PLANS)	,
QUANTITY & SIZE	ATTACH WITH
(2) 2X 6' SPF #2	(2) ROWS 16d @ 8" O.C.
(2) 2X 8' SPF #2	(3) ROWS 16d 9 8" O.C.
(2) 2X 10' SPF #2	(3) ROWS 16d @ 8" O.C.
(2) 2X 12' SPF #2	(3) ROWS 16d @ 8" O.C.
(2) 2X 12' SPF #2	(3) ROWS 16d @ 8" O.C.

2X6 walls use same specs as above, headers to be (3) 2x 3,8,10, or 12 with $1/2^*$ ALL BEAMS SHALL BE BUILT UP WITH 1/2" PLYWOOD FILER. 3. TOP PLATE SPLICES, IF NECESSARY, SHALL BE LOCATED @ 1/4 OF THE LENGTH OF THE BEAM BETWEEN SUPPORTS,

4. TCP PLATES, NAIL 1ST TOP PLATE TO BRG. STUDS W/ (2) 164 © EACH STUD AT HEADERS NAIL (2) ROWS 164 © 10" O.C, STAGGERED, NAIL SECOND TOP PLATE TO FIRST WITH (2) ROWS 164 NAILS & 8" O.C. STAGGERED, LAP ALL.

HEADER STUD REQUIREMENTS (WALL HEIGHT TO 10'-0")

VS .	QUANTITY	ATTACH WITH	
6'-0"	(2) FULL LENGTH & (1) HDR. STUD	(2) ROWS 16d @ 8" O.C.	,
10 18'-0"	(3) FULL LENGTH & (2) HOR. STUDS	(2) ROWS 16d @ 8" O.C.	
IEADER	STUD REQUIREMENTS	(WALL HEIGHT OVER 10	0'- 0

EADER	STUD REQUIREMENTS	(WALL	HEIGHT	OVER	10'- 0")
S	QUANTITY	ATTACH '			
6'-0"	(2) FULL LENGTH & (1) HDR. STUD	(2) ROWS	16d 🏶 8"	O.C.	

- 6'-0"	(2) FULL LENGTH & (1) HDR. STUD	(2) ROWS 16d @ 8" O.C.
-0" TO 12'-0".	(3) FULL LENGTH & (2) HDR, STUDS	(2) ROWS 18d @ 8" O.C.
-0" 10 18'-0"	(4) FULL LENGTH & (2) HDR. STUDS	(2) ROWS 164 @ 8° O.C.

SHING	LE/METAL ROOF ASSE	MRLY INFORMATION
WALKET W.		HINNESON & NA 12 CONTRACTOR OF

ASPHALT SHINGLES:

DESIGNED & APPROVED BY MFG. SPECIFICATIONS FOR WIND ZONES AND TO HAVE SELF—SEAL STRIPS OR BE INTERLOCKING AND CONFORM WITH ASIM D255 OR ASIM D 3462

UNDERLAYMENT:
TO CONFORM WITH ASTM D 226, TYPE I OR ASTM D 4869 TYPE

INDERLAYMENT APPLICATION:
FOR SLOPES 4:12 AND ABOVE — MIN. 1 LAYER. START AT EAVE (PARALLEL) PROVIDE
MIN. 2" LAP W/ SUCCESSIVE LAYERS. FASTEN PER MFG. SPECS. NOTE: ALL FASTENERS FOR UNDERLAYMENT MUST BE MIN. 12 GA. X 11" CORROSION RESISTANT ANNULAR RING SHANK OR COIL ROOFING NAILS THROUGH 32 GA. X 11" DIAMETER TIN CAP

Flashing: Minimum 26 ga galvanized x 18° attached at 4° o.C.

MIN. 26 GA. GALVANIZED STEEL AND OVERLAP A MIN. OF 3" DRIP EDGE TO EXTEND 1/2" BELOW SHEATHING AT EAVES AND EXTEND BACK TO THE ROOF MIN. 2" DRIP EDGE TO BE FASTENED W/ MIN.123 GA. CORROSION RESISTANT ANNULAR RING SHANK NAILS AT 4" O.C. MAX.

(DESIGNED FOR 20 PSF LIVE, 20 PSF DEAD LOADS)
7/16" THICK EXPOSURE I STRUCTURAL PANEL SHEETING PLYWOOD OR OSB PANELS
WITH A MIN. APA SPAN RATING OF 24/16 APPLIED PERPENDICULAR OVER 2 OR MORE
SUPPORTS AT 24" O.C. EDGE CLIPS ARE TO BE CENTERED BETWEEN SUPPORTS. RIDGES,
VALLEYS, OFF RIDGE VENTS, AND SKYLITES REQUIRE 2" X 4" BLOCKING. BLOCKING NOT
REQUIRED FOR OPENINGS 12" OR LESS (NAIL BLOCKING W/(2) 16d NAILS EACH END MIN.)

MECHANICAL NOTES:

2. A 4" CLEARANCE IS TO BE MAINTAINED AROUND ALL AIR HANDLING UNITS OR AS REQ. BY MFG.

4. A/C UNITS MAY HAVE CONC. PAD OR OTHER APPROVED MATERIAL EXTENDING ABV. ADJOINING GRADE OR SHALL BE SUSPENDED A MIN. OF 6" ABV. ADJOINING GRADE. PADS SHALL BE SIZED AS NOTED ON PLANS AND ADJUSTED BASED UPON PRODUCT REQUIREMENTS.

ELECTRICAL NOTES

1. CONTRACTOR SHALL VERIFY WITH FPML THE LOCATION OF SERVICE AND SHALL LOCATE METER & PANELS AS REQUIRED.

2. ALL WIRE SHALL BE THHN COPPER, UNLESS NOTED OTHERWISE. 3. WHERE REQUIRED BY OTHER CODES, SERVICE AND FEEDER CONDUCTORS SHALL BE COPPER OF EQUAL AMPACITY.

4. ALL BRANCH CIRCUITS IN RACEWAY OR NON-METALLIC SHEATHED CABLE 6. VERIFY ALL CONDUCTORS AND BREAKERS WITH EQUIPMENT MANUFACTURER SPECIFICATIONS.

. PROVIDE DISCONNECT SWITCH OF SIZE AS REQUIRED BY LOAD AND UNITS. 8. PROVIDE NON-FUSIBLE GENERAL DUTY SAFETY SVITCHES AT AC EQUIP. AND AP PUMPS NOT VISIBLE FROM CIRCUIT BREAKER PANEL AND AS PER MANUFACTURER RECOMMENDATIONS. 9. PROVIDE GFCI PROTECTION FOR ALL BATHROOM, GARAGE, EXTERIOR, KITCHEN, LAUNDRY, AND WET BAR OUTLETS PER NEC 210.8(A) AND (B).

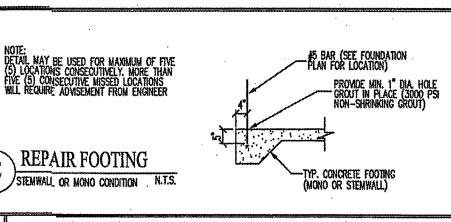
10. ELECTRICAL FIXTURES, TRIM, AND APPLIANCES SHALL BE UL APPROVED AND AS SELECTED BY OWNER. 11. PROVIDE PRE-WIRED TELEPHONE OUTLETS AS SHOWN ON PLAN.

12. PROVIDE PRE-MIRED TV OUTLETS FOR CABLE AS SHOWN ON PLANS. 13. DEDICATED CIRCUITS SHALL HAVE NO MORE THAN 6 DUPLEX RECEPTACLES.

16, PREVINE FOR GARAGE DOOR OPENER SENSOR.

17. PROVIDE ARC FAULT CIRCUIT INTERRUPTER PROTECTION IN DWELLING UNIT LIVING, DINING, FAMILY ROOMS, DENS/LIBRARIES, BEDROOMS, SUNROOMS, CLOSETS, HALLWAYS OR SIM. ROOMS PER NEC 210—12 (B)

20. PROVIDE CARBON MONOXIDE PROTECTION PER RULE 9B-3.0472 FLORIDA ADMINISTRATIVE CODE



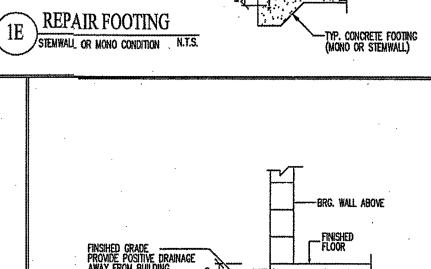
H = AS SPECIFIED ON TYPICAL WALL SECTION OR AS NOTED AT SPECIAL OPENINGS ON FLOOR PLAN

4. TYPICAL STEEL PLACEMENT IS PROVIDED IN THESE DETAILS FOR OPTIONAL STEEL INCREASI AT THE DISCRETION OF THE CONTRACTOR, PLANS SPECIFY "MINIMUM" STEEL REINFORCEMENT.

5. SEE LAP BEND DETAILS FOR STELL BAR LAP REQUIREMENTS AND OPTIONS— DETAIL 1—C THIS PAGE

(OR AS SPECIFIED ON PLAN)

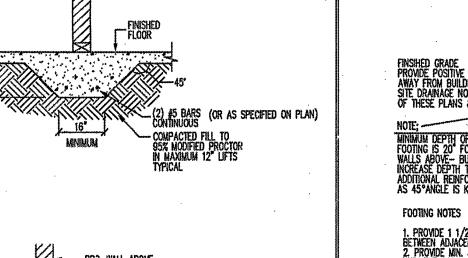
TYPICAL CONCRETE TIE BEAM DETAILS

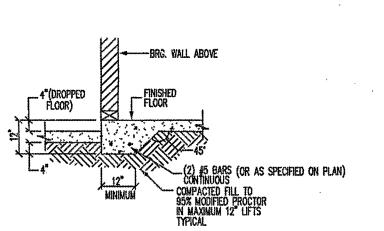


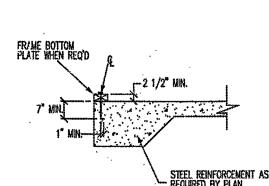
3. Footing concrete shall be Min. 2500 PSI at 28 days 4. Footing reinforcement shall be Min. Grade 40 5. Bearing Capacity of soil for foundations per FBC—R 401

\ TYPICAL FOOTINGS

6. ALL FILL AT FOUNDATIONS AND SLAB IS TO BE PLACED IN MAXIMUM 12" LIFTS AND COMPACTED TO 95% MODIFIED PROCTOR





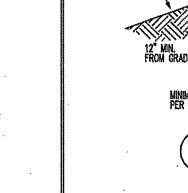


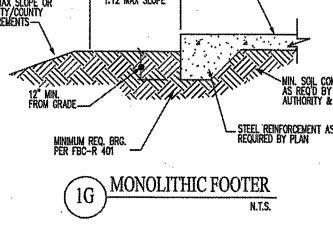
1. EXTERIOR FRAME WALLS SHALL HAVE 1/2" DIAM, MIN, ANCHOR BOLTS W/ 2" MIN, DIA. WASHERS, BOLTS SPACED © 24" O.C. MAX AND WITHIN 6" OF SPLICES UNLESS OTHERWISE NOTED. 2. 5/8" X 4 1/4" RAMSET/REDHEAD (HIN-5842) HEX NUT DYNABOLT SLEEVE ANCHOR MAY BE USED IN PLACE OF "J" BOLTS IN MONOLITHIC FOOTINGS PROVIDED WITH MIN. 3 3/4" EDGE SPACE AND MIN. 2" EMBEDMENT WITH A MIN. 2" DIA WASHER

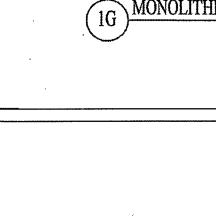
\ ANCHOR BOLT DETAIL MONO EMBEDMENT

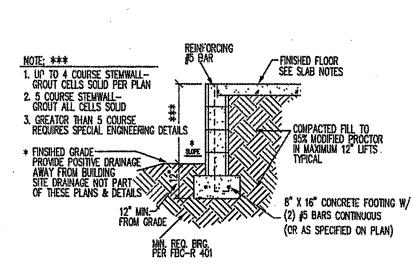
*"NOTICE OF TERMITE PROTECTION" PER FBC

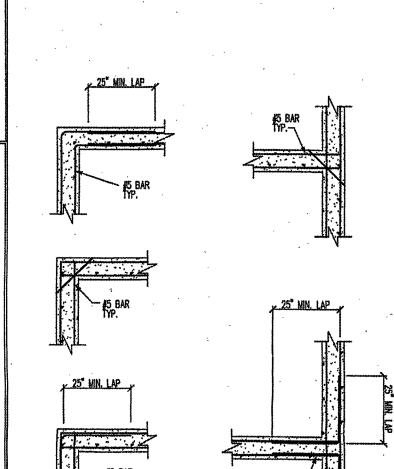
CONCRETE FLOOR SLAB SPECIFICATION



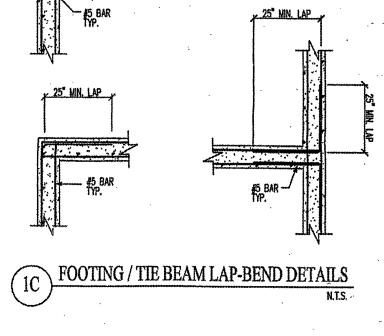








#3 STIRRUPS STARTING @ 3" FROM EACH END THEN @ 6" O.C. ACROSS BEAM



STRUCTURAL DETAILS INDEX

A - TYPICAL CONCRETE TIE BEAM DETAILS

A - MASONRY ARCH WINDOW DETAIL
B - WINDOW-AND PASS THRU DOOR W/OPT, SHIM/FURR
C - ARCH FRAMING
D - MASONRY WINDOW ATTACHMENT DETAIL
E - GARAGE DOOR ATTACHMENT & OPG. DETAIL

A - POST DETAIL B - STEEL POST DETAIL

A - ENTRY DOOR DETAIL

A - KNEEWALL ON TRUSS DETAIL B - FRAME KNEE WALL DETAIL

A - ANGLED FRAME WALL DETAIL B - ANGLE WALL TO MASONRY DETAIL C - INSWING ENTRY DOOR BUCK DETAIL

- WOOD POCKET DOOR DETAIL - FRAMED BRG. WALL (INTERIOR/EXTERIOR) - EXTERIOR FRAME WALL

A - FRAME WALL PERP. TO TRUSSES (NON-BRG) B - FRAME WALL PARALLEL TO TRUSS (BRG. WALL) C - FRAME WALL PARALLEL TO TRUSS (NON-BRG)

A — GABLE END CONNECTION DETAIL B — "KING POST" ONLY GABLE END BRACED TO BEAM DETAIL C — END WALL DETAIL

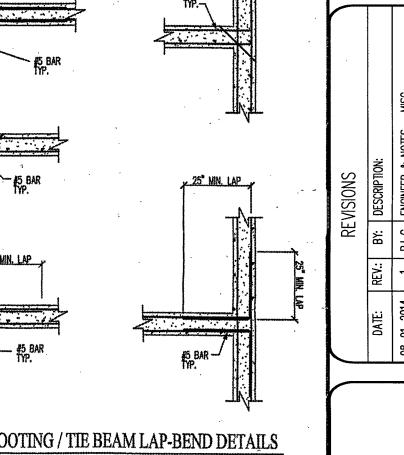
SHEET 1 OF 2

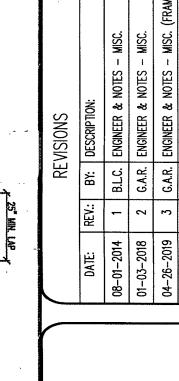
GROUP 2:

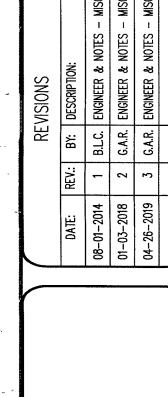
GROUP 3:

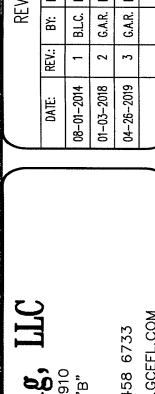
GROUP 4:

SHEET 2 OF 2

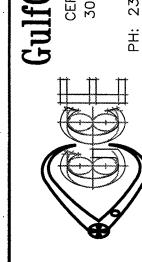


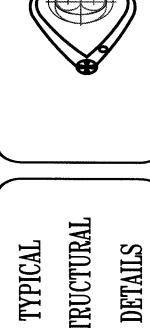






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STRUCTURAL TYPICAL

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