

GENERAL NOTES:

1. THE STRUCTURAL DRAWINGS MUST BE USED IN CONJUNCTION WITH THE ARCHITECTURAL DRAWINGS. THE CONTRACTOR MUST VERIFY THE REQUIREMENTS OF OTHER TRADES AS TO SLEEVES, CHASES, HANGERS, INSERTS, ANCHORS, HOLES, AND ADDITIONAL ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORK.
2. THE STRCUTURAL REPAIRS TO THIS STRUCTURE HAVE BEEN DESIGNED IN ACCORDANCE WITH THE PROVISIONS OF THE NORTH CAROLINA STATE EXISTING BUILDING CODE, 2018 EDITION.
3. PROJECT SCOPE CONSISTS OF STRUCTURAL STABILIZATION OF THE EXISTING BUILDING INCLUDING LIKE-KIND REPLACEMENT OF DAMAGED OR DETERIORATING STRUCTURAL MEMBERS AND REPAIRS OBSERVED BUILDING DISTRESS IN ACCORDANCE WITH THE 2018 NORTH CAROLINA EXISTING BUILDING CODE REPAIRS. SCOPE DOES NOT INCLUDE A COMPREHENSIVE ANALYSIS OR UPGRADE OF THE EXISTING STRUCTURE TO MEET CURRENT CODE REQUIREMENTS.
4. BEFORE PROCEEDING WITH WORK WITHIN THE EXISTING STRUCTURE, THE CONTRACTOR MUST BECOME FAMILIAR WITH THE EXISTING STRUCTURAL CONDITIONS. ANY SHORING OR BRACING SHOWN IS A PARTIAL AND SCHEMATIC REPRESENTATION OF THAT REQUIRED. THE CONTRACTOR MUST BE SOLELY RESPONSIBLE FOR THE DESIGN AND ERECTION OF ANY AND ALL SAFEGUARDS NECESSARY TO PROTECT THE EXISTING STRUCTURE. THE CONTRACTOR MUST PROVIDE SHORING, BRACING, AND OTHER SAFEGUARDS TO MAINTAIN ALL PARTS OF THE STRUCTURE IN A SAFE CONDITION AT ALL TIMES DURING THE PROCESS OF DEMOLITION AND CONSTRUCTION.
5. THE CONTRACTOR MUST FIELD VERIFY THE DIMENSIONS, ELEVATIONS, AND OTHER REQUIREMENTS NECESSARY FOR THE PROPER CONSTRUCTION AND ALIGNMENT OF THE NEW PORTIONS OF THE STRUCTURE TO THE EXISTING. ANY DIMENSIONS SHOWN OF EXISTING STRUCTURES MUST BE CONSIDERED AS APPROXIMATE AND ADEQUATE FOR BIDDING PURPOSES ONLY. THE CONTRACTOR MUST MAKE ALL MEASUREMENTS NECESSARY FOR THE FABRICATION AND ERECTION OF STRUCTURAL MEMBERS. DISCREPANCIES MUST BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER.

6. DESIGN CRITERIA:

CLASSIFICATION OF BUILDING	
RISK CATEGORY	III
LIVE LOADS - UNIFORM:	
FLOOR	100 PSF
ROOF	20 PSF
LIVE LOADS - CONCENTRATED:	
FLOORS	1,000#
ROOFS	300#

UNLESS OTHERWISE NOTED, CONCENTRATED LOADS ARE APPLIED UNIFORMLY OVER 2'-6" x 2'-6" AREA.

SNOW LOADS:

GROUND SNOW LOAD	10 PSF
SLOPED ROOF LOAD	11 PSF
IMPORTANCE FACTOR (Is)	1.1
THERMAL FACTOR (Ct)	1.0
EXPOSURE FACTOR (Ce)	1.0

WIND LOADS:

ULTIMATE DESIGN WIND SPEED (VULT)	128 MPH
NOMINAL DESIGN (VASD) WIND SPEED	103 MPH
EXPOSURE CATEGORY	B
INTERNAL PRESSURE COEFFICIENT	±0.18
COMPONENT AND CLADDING PRESSURES:	
WALLS, ZONE 5 (10 SF)	37 PSF
ROOF, ZONE 3 (10 SF)	76 PSF
WIND BASE SHEAR (X)	27 KIPS
WIND BASE SHEAR (Y)	33 KIPS

SEISMIC LOADS:

SITE CLASSIFICATION	D (PRESUMPTIVE)
SEISMIC DESIGN CATEGORY	B
IMPORTANCE FACTOR (IE)	1.25
SPECTRAL RESPONSE ACCELERATIONS:	
Ss	0.100
S1	0.052
Sms	0.160
S1m1	0.125
Sps	0.107
S1p1	0.084
ANALYSIS PROCEDURE	
EQUIVALENT LATERAL FORCE	
LATERAL FORCE RESISTING SYSTEM	LIGHT-FRAMED WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR
RESPONSE MODIFICATION COEFFICIENT (R)	6.5
SEISMIC RESPONSE COEFFICIENT (Cs)	0.0205
SEIMISIC BASE SHEAR	10.KIPS
CONTROLLING LATERAL LOAD	WIND

FOUNDATION NOTES:

1. FOUNDATIONS HAVE BEEN DESIGNED FOR A PRESUMPTIVE NET ALLOWABLE SOIL BEARING PRESSURE OF 1500 PSF.
2. PRIOR TO PLACING FOUNDATION CONCRETE, ALL FOUNDATION EXCAVATIONS MUST BE INSPECTED BY THE OWNER'S GEOTECHNICAL TESTING AGENCY TO EXPLORE THE EXTENT OF LOOSE, SOFT, EXPANSIVE, OR OTHERWISE UNSATISFACTORY SOIL MATERIAL AND TO VERIFY DESIGN BEARING PRESSURE. DIRECTION FOR CORRECTIVE ACTION WILL BE PROVIDED BY THE OWNER'S GEOTECHNICAL TESTING AGENCY WHERE UNSATISFACTORY SOILS ARE PRESENT.

STRUCTURAL STEEL NOTES:

1. STRUCTURAL STEEL MUST BE IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 360.
2. STRUCTURAL STEEL MUST COMPLY WITH THE FOLLOWING SPECIFICATIONS:

A. STRUCTURAL STEEL SHAPES, PLATES AND BARS - ASTM A 36, Fy = 36 KSI

B. HOLLOW STRUCTURAL SECTIONS (HSS) - SQUARE AND RECTANGULAR - ASTM A 500, GRADE C, Fy = 50 KSI

C. TIE RODS: A36 RODS WITH THREADED ENDS WITH TURNBUCKLES AND CLEVISES RATED FOR FULL TENSION CAPACITY OF ROD
3. WELDING MUST BE IN ACCORDANCE WITH AWS D1.1, "STRUCTURAL WELDING CODE - STEEL." WELD ELECTRODES MUST BE E70XX LOW HYDROGEN. UNLESS OTHERWISE NOTED, PROVIDE CONTINUOUS FILLET WELDS WITH MINIMUM SIZE REQUIRED BY TABLE J2.4 AISC 360.
4. HOT-DIP GALVANIZE AFTER FABRICATION THE FOLLOWING:

A. ANGLES AND PLATES SUPPORTING MASONRY IN EXTERIOR WALLS.

B. LINTELS AND LINTEL ASSEMBLIES SUPPORTING MASONRY IN EXTERIOR WALLS.

C. ALL STEEL EXPOSED TO WEATHER IN THE FINAL CONSTRUCTION.

D. ITEMS IDENTIFIED AS GALVANIZED ON ARCHITECTURAL OR STRUCTURAL DRAWINGS.

ROUGH CARPENTRY NOTES:

1. ROUGH CARPENTRY MUST BE IN ACCORDANCE WITH THE AMERICAN WOOD COUNCIL (AWC) "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION."
2. UNLESS OTHERWISE NOTED, USE 'COMMON' NAILS AND ALL NAILING MUST CONFORM TO THE "FASTENING SCHEDULE" TABLE 2304.10.1 OF THE BUILDING CODE.
3. WOOD FRAMING MEMBERS MUST COMPLY WITH PS 20 "AMERICAN SOFTWOOD LUMBER STANDARD" AND THE FOLLOWING REQUIREMENTS:

A. MOISTURE CONTENT - SEASONED, WITH 19 PERCENT MAXIMUM MOISTURE CONTENT.

B. GRADE - NO. 2, OR BETTER UNLESS OTHERWISE NOTED.

C. SPECIES - SPRUCE-PINE-FIR UNDER NLGA RULES.
4. WOOD STRUCTURAL PANELS (WSP) MUST COMPLY WITH PS 1 "U.S. PRODUCT STANDARD FOR CONSTRUCTION AND INDUSTRIAL PLYWOOD" FOR PLYWOOD CONSTRUCTION PANELS AND THE FOLLOWING REQUIREMENTS:

A. INTERIOR WALL AND SHEARWALL SHEATHING: 15/32 INCH, APA RATED SHEATHING, EXPOSURE 1 DURABILITY CLASSIFICATION. EXTERIOR WALL AND SHEARWALL SHEATHING: 19/32 INCH, APA RATED SHEATHING, EXPOSURE 1 DURABILITY CLASSIFICATION.

B. FLOOR SHEATHING: 23/32 INCH, TONGUE AND GROOVE, APA RATED SHEATHING, EXPOSURE 1 DURABILITY CLASSIFICATION.

C. ROOF SHEATHING: 9/16 INCH, APA RATED SHEATHING, EXPOSURE 1 DURABILITY CLASSIFICATION. PROVIDE TONGUE-AND-GROOVE EDGES OR USE "PLY-CLIPS" AT MID-SPAN BETWEEN EACH SUPPORT.
5. ALL WOOD FRAMING MEMBERS PERMANENTLY EXPOSED TO THE WEATHER AND ALL SILL PLATES OR TOP PLATES IN CONTACT WITH CONCRETE OR MASONRY MUST BE PRESERVATIVE-TREATED.
6. STEEL PLATE CONNECTORS MUST COMPLY WITH ASTM A 36 SPECIFICATIONS (Fy= 36 KSI). BOLTS CONNECTING WOOD MEMBERS MUST COMPLY WITH ASTM A 307 COMMON STEEL BOLTS, AND MUST BE 3/4 INCH DIAMETER, UNLESS OTHERWISE NOTED.
7. METAL FRAMING ANCHORS, HOLD DOWNS, HURRICANE TIES, HANGERS, ETC. MUST COMPLY WITH ASTM A 653 AND BE CAPABLE OF SUPPORTING THE REACTIONS SHOWN. WHERE PRODUCTS OF A SPECIFIC MANUFACTURER ARE SHOWN, EQUAL PRODUCTS OF ANOTHER MANUFACTURER MAY BE USED IF APPROVED BY ENGINEER.
8. UNLESS OTHERWISE NOTED, ATTACH BLOCKING AND NAILERS TO STEEL FRAMING USING 3/16 INCH DIAMETER POWDER ACTUATED FASTENERS AT 24 INCHES ON CENTER OR 1/2 INCH DIAMETER BOLTS AT 48 INCHES ON CENTER. STAGGER FASTENERS TO ALTERNATE SIDES OF BEAM WEB.
9. ALL CONNECTION HARDWARE IN CONTACT WITH PRESERVATIVE TREATED WOOD MUST BE HOT-DIP GALVANIZED COATED.
10. POWDER ACTUATED FASTENERS (PAF) MUST HAVE A MINIMUM ALLOWABLE CAPACITY INTO THE BASE MATERIAL AS FOLLOWS UNLESS OTHERWISE NOTED:

A. STEEL: SHEAR = 600 LBS
TENSION = 250 LBS

B. CONCRETE: SHEAR = 260 LBS
TENSION = 255 LBS

POST-INSTALLED ANCHOR NOTES:

1. ALL POST INSTALLED ANCHORS INDICATED ON THE DRAWINGS ARE BY HILTI, INC. AND MUST BE CONSIDERED THE BASIS OF DESIGN PRODUCT. WHERE NOT EXPLICITLY INDICATED IN THE DRAWINGS, THE FOLLOWING ANCHORS/ADHESIVES MUST BE USED:

A. ANCHORAGE TO CONCRETE

1. ADHESIVE ANCHORS FOR CRACKED AND UNCRACKED CONCRETE USE:

a. HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT (TE-CD OR TE-YD) AND VC 20/40 VACUUM SYSTEM (VC 20-U OR VC40U) WITH STEEL THREADED ROD PER ICC ESR-3187.

2. SCREW ANCHORS FOR CRACKED AND UNCRACKED CONCRETE USE:

a. HILTI KWIK HUS EZ SCREW ANCHORS PER ICC ESR-3027.

B. REBAR DOWELING INTO CONCRETE

1. ADHESIVE ANCHORS FOR CRACKED AND UNCRACKED CONCRETE USE:

a. HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT (TE-CD OR TE-YD) AND VC 20/40 VACUUM SYSTEM (VC 20-U OR VC 40-U) WITH CONTINUOUSLY DEFORMED REBAR PER ICC ESR-3187.

C. ANCHORAGE TO SOLID GROUTED MASONRY

1. ADHESIVE ANCHORS USE:

a. HILTI HIT-HY 270 MASONRY ADHESIVE ANCHORING SYSTEM (ICC PENDING).

b. STEEL ANCHOR ELEMENT MUST BE HILTI HAS-E CONTINUOUSLY THREADED ROD.

2. MECHANICAL ANCHORS USE:

a. HILTI KWIK HUS EZ SCREW ANCHORS PER ICC ESR 3056.

D. ANCHORAGE TO HOLLOW / MULTI-WYTHE MASONRY

1. ADHESIVE ANCHORS USE:

a. HILTI HIT-HY 270 MASONRY ADHESIVE ANCHORING SYSTEM PERICCESR-3342.

b. STEEL ANCHOR ELEMENT MUST BE HILTI HAS-E CONTINUOUSLY THREADED ROD OR CONTINUOUSLY DEFORMED STEEL REBAR.

c. THE APPROPRIATE SIZE SCREEN TUBE MUST BE USED PER ADHESIVE MANUFACTURER'S RECOMMENDATION.

2. ALTERNATE POST INSTALLED ANCHOR PRODUCTS MAY BE SUBMITTED TO THE ENGINEER FOR REVIEW AND POSSIBLE APPROVAL. ALL SUBSTITUTION REQUESTS MUST BE ACCOMPANIED BY AN ICC ESR SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR SEISMIC USES, LOAD RESISTANCE, INSTALLATION CATEGORY, AND COMPREHENSIVE INSTALLATION INSTRUCTIONS. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE AND INSTALLATION TEMPERATURE. ALTERNATE PRODUCTS MAY REQUIRE MODIFICATIONS TO ANCHOR DIAMETER, SPACING, AND EMBEDMENT.

3. INSTALL ANCHORS PER THE MANUFACTURER INSTRUCTIONS, AS INCLUDED IN THE ANCHOR PACKAGING.

4. THE CONTRACTOR MUST ARRANGE FOR AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF ANCHOR INSTALLATION.

5. ANCHOR CAPACITY IS DEPENDANT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.

6. EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. UNLESS NOTED ON THE DRAWINGS THAT THE BARS CAN BE CUT, THE CONTRACTOR MUST LOCATE THE POSITION OF THE REINFORCING BARS AT THE LOCATIONS OF THE CONCRETE ANCHORS, BY FERROSCAN OR GPR.

7. ALL POST INSTALLED ANCHORS REQUIRE CONTINUOUS SPECIAL INSPECTIONS TO VERIFY INSTALLATION HAS BEEN PERFORMED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS. REFERENCE THE STATEMENT AND SCHEDULE OF SPECIAL INSPECTIONS FOR ADDITIONAL INFORMATION.
- DEMOLITION NOTES:
1. SOME TERMS INDICATED ON PLAN ARE DEFINED AS FOLLOWS:

A. REMOVE: DETACH ITEMS FROM EXISTING CONSTRUCTION AND LEGALLY DISPOSE OF THEM OFF-SITE.

B. REMOVE AND SALVAGE: DETACH ITEMS FROM EXISTING CONSTRUCTION AND DELIVER THEM TO THE OWNER READY FOR REUSE.

C. REMOVE AND REINSTALL: DETACH ITEMS FROM EXISTING CONSTRUCTION, PREPARE THEM FOR REUSE, AND REINSTALL THEM WHERE INDICATED.

D. EXISTING TO REMAIN: EXISTING ITEMS OF CONSTRUCTION THAT ARE NOT TO BE REMOVED.

2. COMPLY WITH LOCAL NOISE, DUST AND EROSION CONTROL REGULATIONS. CONTROL DUST FROM DEMOLITION TO PREVENT IT FROM SPREADING TO OCCUPIED PORTIONS OF BUILDING AND TO AVOID CREATING A NUISANCE IN SURROUNDING AREA.

3. OBTAIN REQUIRED PERMITS FROM GOVERNING AUTHORITIES.

4. PROVIDE TEMPORARY BARRICADES AND OTHER PROTECTION REQUIRED TO PREVENT INJURY TO PEOPLE AND DAMAGE TO ADJACENT PROPERTY.

5. AT END OF EACH WORKDAY AND DURING INCLEMENT WEATHER, COVER AND PROTECT AREAS OF OPENED UP AND UNFINISHED WORK WITH WEATHER PROOF BARRIERS, AS REQUIRED.

6. PROTECT FROM DAMAGE EXISTING ROADS, WALKS, CURBS, LANDSCAPE, AND OTHER SITE AND BUILDING STRUCTURES. REPAIR OR REPLACE DAMAGED ITEMS.

7. REMOVE MATERIAL RESULTING FROM DEMOLITION OPERATIONS, EXCEPT AS OTHERWISE INDICATED, AND DISPOSE OF IN ACCORDANCE WITH APPLICABLE LAWS AND REGULATIONS AS PART OF THE WORK. CONTROL RUBBISH, DEBRIS, AND DUST BY APPROVED METHODS, AS REQUIRED BY LOCAL NOISE, DUST, AND EROSION CONTROL REGULATIONS. SOLID WASTE DISPOSAL: IN ACCORDANCE WITH COV 10.1 - 1408.1, (1997) VIRGINIA WASTE MANAGEMENT ACT; ARTICLE 2, SOLID WASTE MANAGEMENT. DISPOSAL OF SOLID WASTE IN OPEN DUMPS IS PROHIBITED.
- THESE DRAWINGS ARE RELEASED FOR THE FOLLOWING USE. ANY OTHER USE OF THESE DRAWINGS IS AT THE RISK OF THE CONTRACTOR OR OTHERS USING THESE DRAWINGS FOR THAT UNAUTHORIZED USE. LYNCH MYKINS IS NOT RESPONSIBLE FOR ADDITIONAL COSTS DUE TO CHANGES, COORDINATION OR ADDITIONAL SCOPE OF WORK REQUIRED DUE TO SUCH UNAUTHORIZED USE.

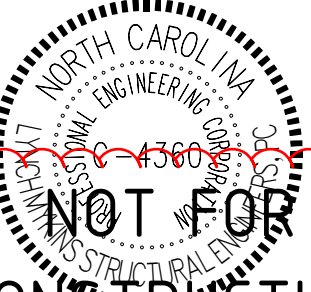
☒	PRELIMINARY BUDGET PRICING
☐	EARLY FOUNDATION PACKAGE
☐	WILL ORDER PACKAGE
☐	EARLY STEEL PACKAGE
☐	PERMIT SET
☐	CONSTRUCTION SET
- 

CALCULATE, COLLABORATE, CREATE.

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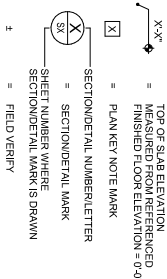
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NOT FOR
CONSTRUCTION
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ONLY
- PROJECT TYPE: REPAIRS

KADESH AME ZION

119 E. GALE ST., EDENTON NC
- | | |
|---------|----------|
| DATE | 10.23.24 |
| DR. | |
| CH. | TM |
| PROJ. # | 20130 |
- | REVISIONS | | |
|-----------|-------------|------|
| NO. | DESCRIPTION | DATE |
- GENERAL NOTES
- S001

PLAN LEGEND:

[illegible]

DATE	02.12.24
DR.	
CH.	Checker
PROJ. #	20130

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NO.	DESCRIPTION
DATE	

KEY NOTES

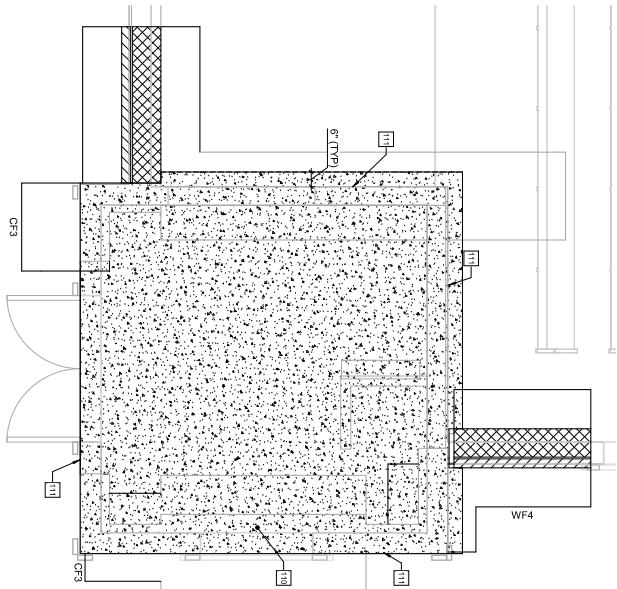
- 110 12" 40M PSI CONCRETE WITH #6@12" O.C. 3' FROM TOP OF SLAB. 1" PERL EXISTING SUBGRADE AND PLACE CONCRETE ON TOP OF GRADE. EXTEND FOUNDATION 6" BEYOND EXISTING WOOD BEAM ABOVE WHERE POSSIBLE. POUR UP EXISTING FOUNDATION WALLS ELSEWHERE.
- 111 EXISTING WOOD BEAM.
- 102 EXISTING 6x6 WOOD POST.

FOUNDATION PLAN NOTES

- A. REFERENCE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO NONBEARING WALLS, WALL CONTROL JOINTS AND OPENINGS.
- B. UNLESS OTHERWISE NOTED, ALL ELEVATIONS ARE BASED ON A FINISHED FIRST FLOOR REFERENCE OF 0'-0". FINISHED FLOOR ELEVATIONS AT EACH LEVEL ARE INDICATED ON PLANS, REFERENCE ARCHITECTURAL DRAWINGS FOR FINISHED FLOOR MATERIALS.
- C. UNLESS OTHERWISE INDICATED, EXTEND WALL FOOTINGS A MINIMUM OF 8 INCHES BEYOND ENDS OF WALLS.
- D. DIMENSIONS SHOWN ON FOUNDATION PLAN ARE TO COLUMN GRIDLINES AND OUTSIDE FACE OF FOUNDATION WALLS, UNLESS OTHERWISE NOTED.

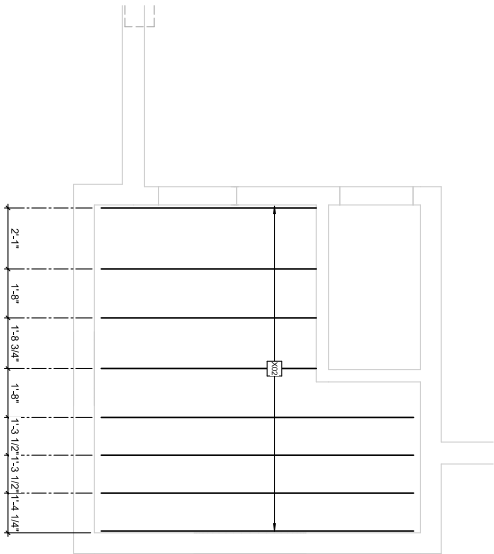
FRAMING PLAN NOTES

- A. REFERENCE FOUNDATION PLAN AND ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN.
- B. TOP OF FINISHED FLOOR ELEVATION MUST BE AS NOTED ON SLAB PLANS.



B1 FOUNDATION PLAN

1/2" = 1'-0"



A1 SECOND FLOOR FRAMING PLAN

1/2" = 1'-0"

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S111

BELL TOWER
LOFT FRAMING
& FOUNDATION
PLAN

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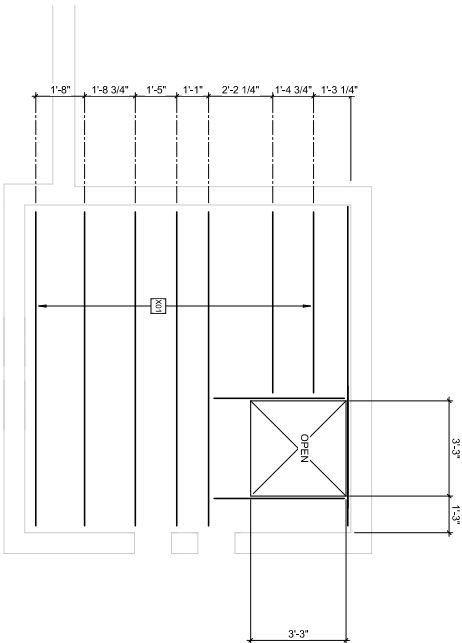
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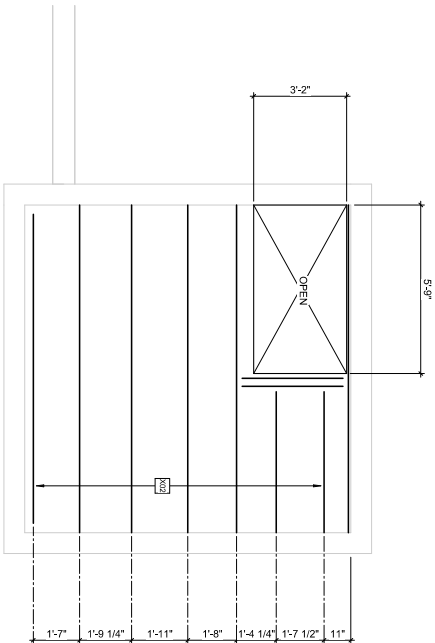


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KEY NOTES
X01 EXISTING 7x7 WOOD BEAM
X02 EXISTING 6x8 WOOD POST



B1 BELFRY UPPER FRAMING PLAN



A1 BELFRY LOWER FRAMING PLAN



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LOWER &
UPPER FRAMING
PLAN

S121

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KEY NOTES

101 2x6 AT 2'-0" ON CENTER.

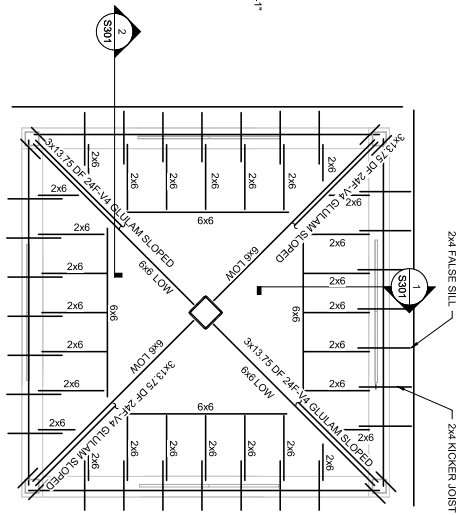
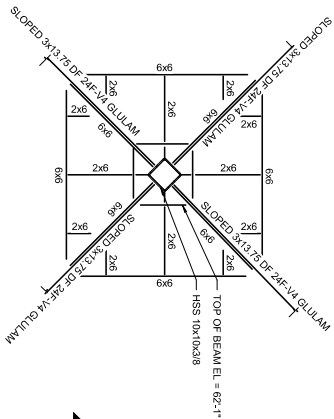
102 4x BLOCKING BETWEEN OUTLOOKERS RIPPED TO BE LEVEL WITH TOPS OF OUTLOOKERS.

X02 EXISTING 6x6 WOOD POST.

X03 EXISTING 2x6 FLOOR FRAMING.

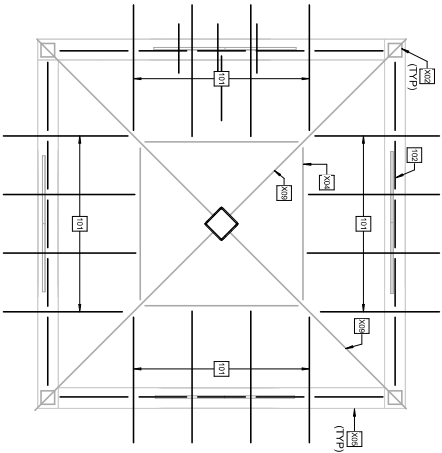
X05 EXISTING 2x6 FLOOR FRAMING.

X09 EXISTING 7x7 BEAM.



B1 UPPER STEEPLE FRAMING
1/2" = 1'-0"

B2 LOWER STEEPLE FRAMING
1/2" = 1'-0"



A1 BELL TOWER SOFFIT FRAMING PLAN
1/2" = 1'-0"



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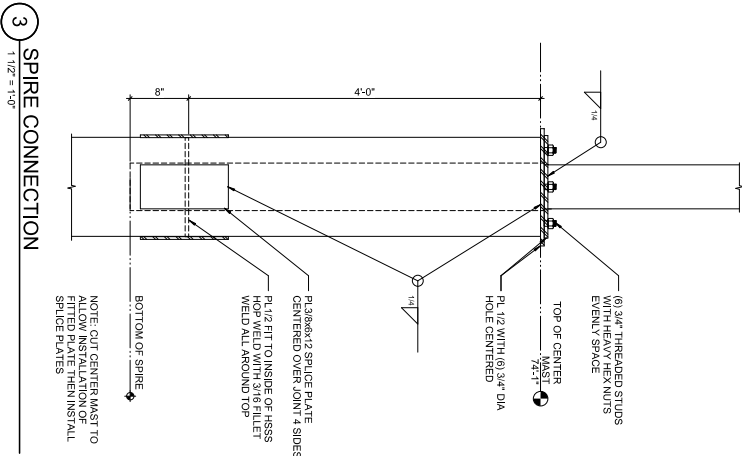
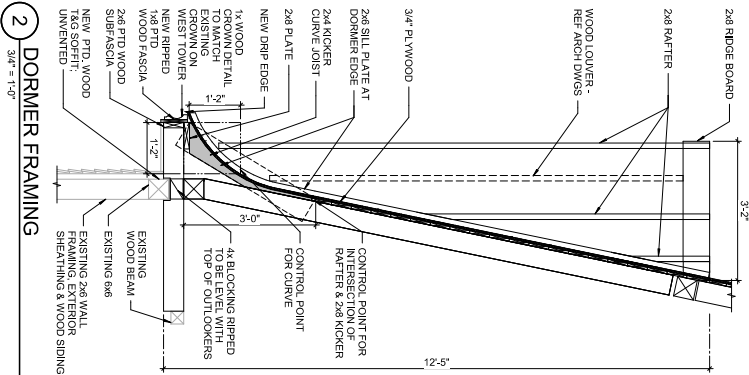
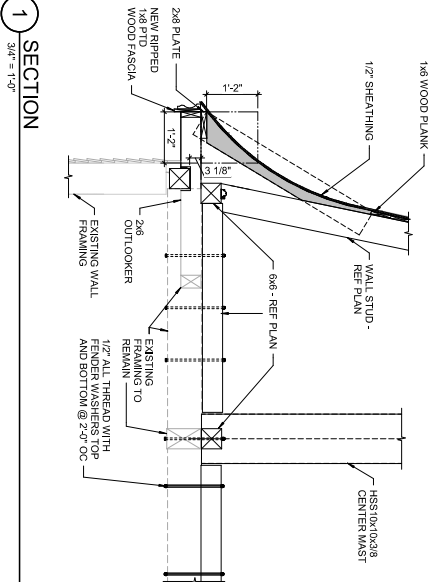
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SPIRE FRAMING
PLAN

S131

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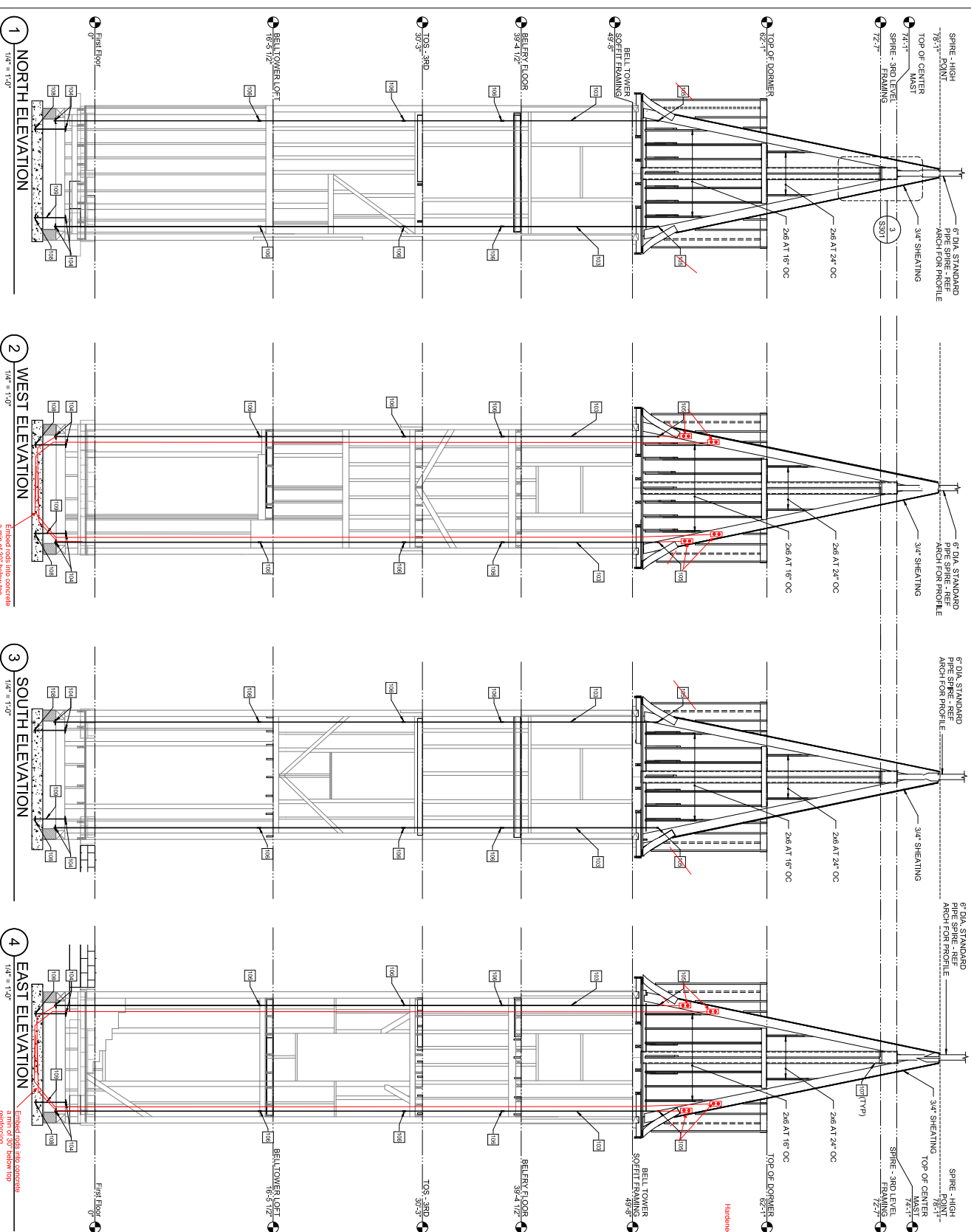
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- KEY NOTES**

108 (2) 5/8" 26# DA STEEL ROD
 DRILL 1" DIA HOLE
 THROUGH EXISTING
 CONCRETE AND
 ALLOW ROD
 INSTALLATION.
 104
 105
 106
 107
 108
 109
- SPIRE - HIGH
 TOP OF CENTER
 MAST
 74'-1"
 SPIRE - 3RD LEVEL
 72'-7"
 3/4" SHEATING
 2x6 AT 24" OC
 2x6 AT 16" OC
 TOP OF DORMER
 62'-1"
 BELL TOWER
 SOFT FLOOR
 49'-5"
 BELL TOWER LOFT
 18'-5 1/2"
 TOS - 3RD
 30'-3"
 BELLERY FLOOR
 39'-4 1/2"
 BELLERY LOFT
 18'-5 1/2"
 First Floor



PROJECT TYPE: REPAIRS

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ELEVATIONS

S401

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